PRILOGA 1C

NASLOVNA STRAN NAČRTA GRADBENIŠTVA

PODATKI O GRADNJI		
naziv gradnje		Dozidava POŠ in vrtca Tabor Inv.: Občina Tabor
kratek opis gradnje		Investitor želi dozidati shrambo, rekonstruirati in spremeniti namembnost iz neizkoriščene podstrehe v igralnico vrtca s sanitarijami, nadzidati in spremeniti namembnost iz neizkoriščene podstrehe šole v prostore za izvajanje pouka.
VRSTE GRADNJE	☐	NOVOGRADNJA - NOVOZGRAJEN OBJEKT
označiti vse ustrezne vrste gradnje	☑	NOVOGRADNJA - PRIZIDAVA
	☑	REKONSTRUKCIJA
	☑	SPREMEMBA NAMEMBNOSTI
	☐	ODSTRANITEV CELOTNEGA OBJEKTA
	☐	LEGALIZACIJA
	☐	MANJŠA REKONSTRUKCIJA
PODATKI O PROJEKTNI DOKUMENTACIJI		
vrsta dokumentacije		PZI (projektna dokumentacija za izvedbo gradnje)
številka projekta		01/2021
PODATKI O NAČRTU		
strokovno področje načrta		2 Načrt s področja gradbeništva
naziv načrta		Konstrukcije
številka načrta		15/25
datum izdelave		avgust 2025
datum spremembe		
PODATKI O PROJEKTANTU NAČRTA		
projektant načrta (naziv družbe)		Šturm, d. o. o.
naslov		Gomilsko 30a, 3303 Gomilsko
odgovorna oseba projektanta načrta		Danilo ŠTURM, univ. dipl. inž. grad.
podpis odgovorne osebe projektanta načrta		ŠTURM d.o.o. Gomilsko 30a, 3303 GOMILSKO 2
PODATKI O IZDELOVALCU NAČRTA		
ime in priimek pooblaščenega arhitekta, pooblaščenega inženirja		Danilo ŠTURM, univ. dipl. inž. grad.
identifikacijska številka		IZS G-0150
podpis pooblaščenega arhitekta, pooblaščenega inženirja		DANILO ŠTURM univ. dipl. inž. grad. IZS G-0150

PRILOGA 2C

IZJAVA PROJEKTANTA NAČRTA IN POOBLAŠČENEGA STOKOVNJAKA, KI JE IZDELAL NAČRT V PZI IN PID

PROJEKTANT NAČRTA	
projektant načrta (naziv družbe)	Štum, d. o. o.
naslov	Gomilsko 30a, 3303 Gomilsko
odgovorna oseba projektanta načrta	Danilo ŠTURM, univ. dipl. inž. grad.

IN POOBLAŠČENI STROKOVNJAK, KI JE IZDELAL NAČRT	
pooblaščeni strokovnjak	Danilo ŠTURM, univ. dipl. inž. grad.

IZJAVLJAVA:

da načrt

vrsta dokumentacije	PZI (projektna dokumentacija za izvedbo gradnje)
strokovno področje načrta	2 Načrt s področja gradbeništva
naziv načrta	Konstrukcije - POŠ Tabor
številka načrta	15/25
datum izdelave	avgust 2025

upošteva relevantne predpise in druge normativne dokumente ter da so upoštevane ustrezne bistvene in druge zahteve.

pooblaščeni strokovnjak	Danilo ŠTURM, univ. dipl. inž. grad.
identifikacijska številka	IZS G-0150
podpis pooblaščenega strokovnjaka	DANILO ŠTURM univ. dipl. inž. grad. IZS G-0150
odgovorna oseba projektanta načrta	Danilo ŠTURM, univ. dipl. inž. grad.
podpis odgovorne osebe projektanta načrta	ŠTURM d.o.o. Gomilsko 30a, 3303 GOMILSKO 2

2.00 TEHNIČNO POROČILO

2.01 STATIČNO POROČILO

1. Splošno:
Investitor namerava izkoristiti del podstrešja za igralnico vrtca (b), prostor v pritličju za shrambo (c) in povišati šolo za eno etažo (a). Na vzhodu se doda požarno stopnišče (a2) in na vezni del klimatska naprava (a3).

2. Konstrukcija:

a - Po kontroli obstoječih temeljev je nadvišanje za eno etažo mogoče le z uporabo lahkih materialov in konstrukcij. Za ostrešje je izbrana palična lesena konstrukcija in pločevinasta kritina, lahki estrih in čim lažji zidovi z upoštevanjem zvočne izolacije. Skenirana je bila armatura AB plošče – ustreza projektu. Nosilna konstrukcije nadzidave je AB skelet z zidanimi polnili. Stebre je potrebno sidrati v obstoječo arm. bet. konstrukcijo.
a2 - Stopnišče je varjena in vijačena vroče cinkana jeklena konstrukcija. Podesti in stopnje so montažne (tipske), vijačene na robne nosilce UNP 200.
a3 – Klimatska naprava se montira na jekleno vroče cinkano podkonstrukcijo iz dveh profilov HEA 200. Sidrana sta v atiko veznega objekta.

b - Zaradi min. dodatne obtežbe temeljev je potrebno uporabiti najlažje materiale (kritina, zidovi, estrih). Stebri in nosilni zidovi morajo biti nad obstoječimi v pritličju! Le predelni zidovi iz lahkih materialov so lahko drugje. Vmesna lega in steber sta zaradi večjega razpona iz jekla. Vertikalne vezi sidrati v konstrukcijo.

c – Konstrukcija shrambe je v celoti lesena (špirovci, lege, sohe). Kritina je lahka.

Jeklene konstrukcije: Detajle spajanja in montažni načrt pripravi izvajalec konstrukcije glede na svojo mehanizacijo.

3. Vplivi na konstrukcijo po SIST EN 1990, 1991, 1998
a- sneg
naklon strešine 6 stopinj
nadmorska višina do 330 m, cona A2
$S = \mu_1 * Ce * Ct * Sk = 1,25$ KN/m2
$Sk = 1,55$ KN/m2
$\mu_1 = 0,80$
$Ce = Ct = 1,0$

b- veter
cona I, teren III, Nv < 1600 m, Ho = 10 m
$w = q(ze) * c$
$q = 0,5 * \rho * v_b^2 = 0,39$ KN/m2
$v = 1,0 * 1,0 * 1.0 * 25 = 25$ m/s
$\rho = 1,25$ kg/m3
veter na strešino:
H: $Cpe \leq +0,7$ I: $Cpe \leq -0,4$; $Ce(z) < 1,6$
$w_i = q_b * Ce(z) * c_i$
$w_H = 0,39 * 1,6 * (0,7) = 0,40$ KN/m2 ; $w_I = 0,39 * 1,6 * (-0,4) = -0,25$ KN/m2

c- potres (SIST –EN –1998)
Projektni pospešek tal a = 0,20 g
Ker gre za pomemben javni objekt upoštevam pospešek 0,25 g

d- koristne obtežbe (SIST-EN-1991)

strehe H	splošno	q = 0,4 KN/m2	Q = 1,0 KN
objekti C	splošno	q = 3,0	Q = 4,0

Obtežni primeri so izbrani glede na :
-mejno stanje uporabnosti MSU
-mejno stanje nosilnosti MSN

4. Materiali

beton	C25/30 konstrukcija nad temelji
	C20/25 temelji
armatura	S500-(RA 500/560)
	S500-(MA 500/560)
zaščitni sloj	3,0 cm-temelji
	2,0 cm-ostala konstrukcija
les	žagan les C 24 (2. ktg)
	lepljenci GL 24 h (lepljenci iz ktg. 2)
	lesne plošče OSB – 3

5. Temeljenje
Temeljiti na raščenih, enakomernih tleh v globini min. 80 cm. Upoštevana minimalna nosilnost tal 150 KN/m2.

Danilo Šturm u.d.i.g.

DANILO ŠTURM
univ. dipl. inž. grad.
IZS G-0150

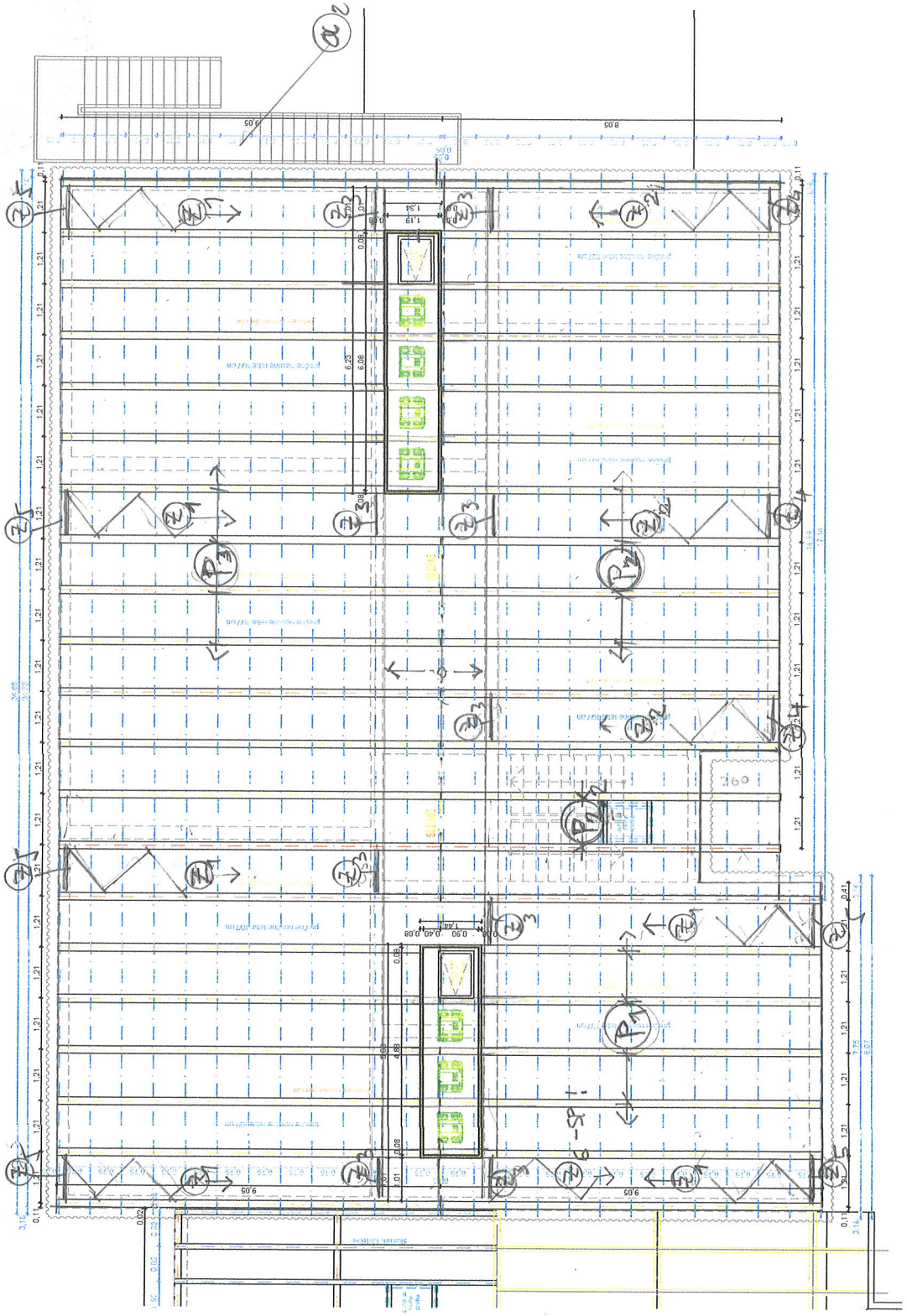
Sledi:
pozicijski načrti
statični izračun
risbe

ŠOLA — a
VRTEC — b
SHRAMBA — c

Q - poticijski

P1 1:1

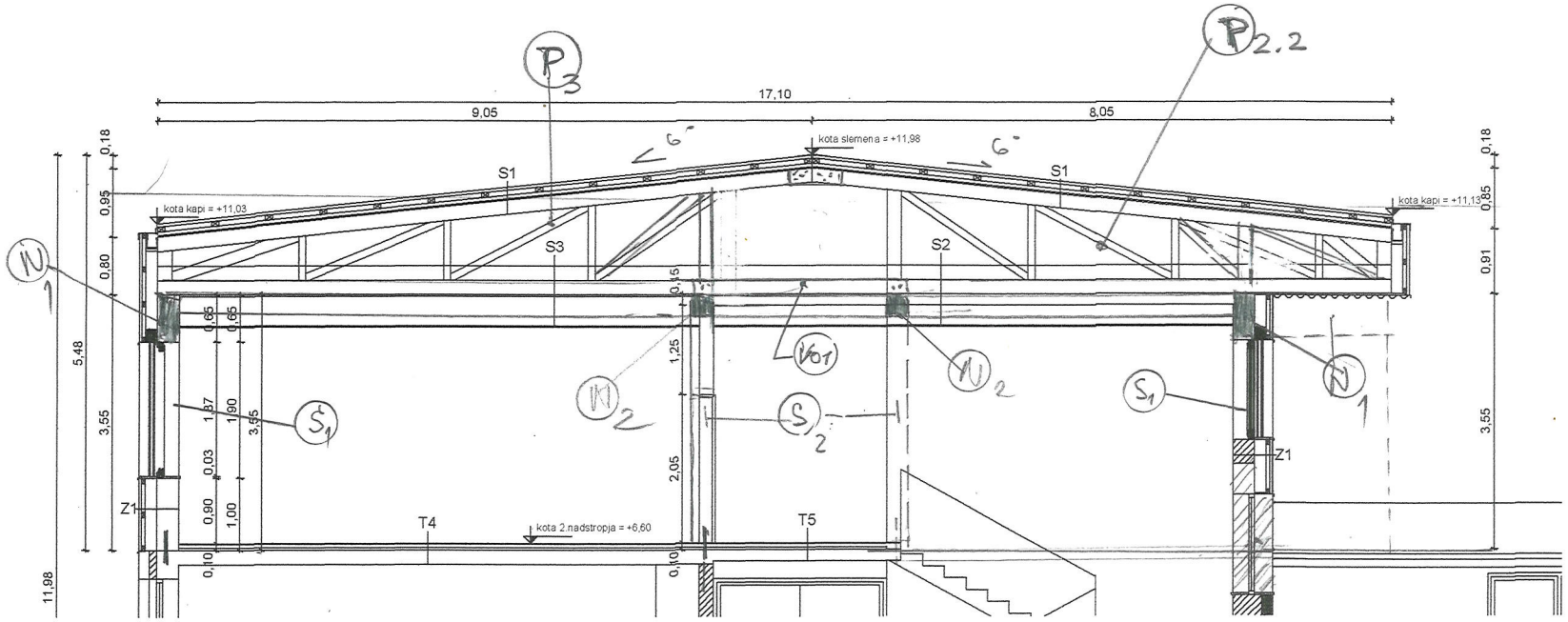
OSTREŠJE 1:150



Pozicijski N.

P1

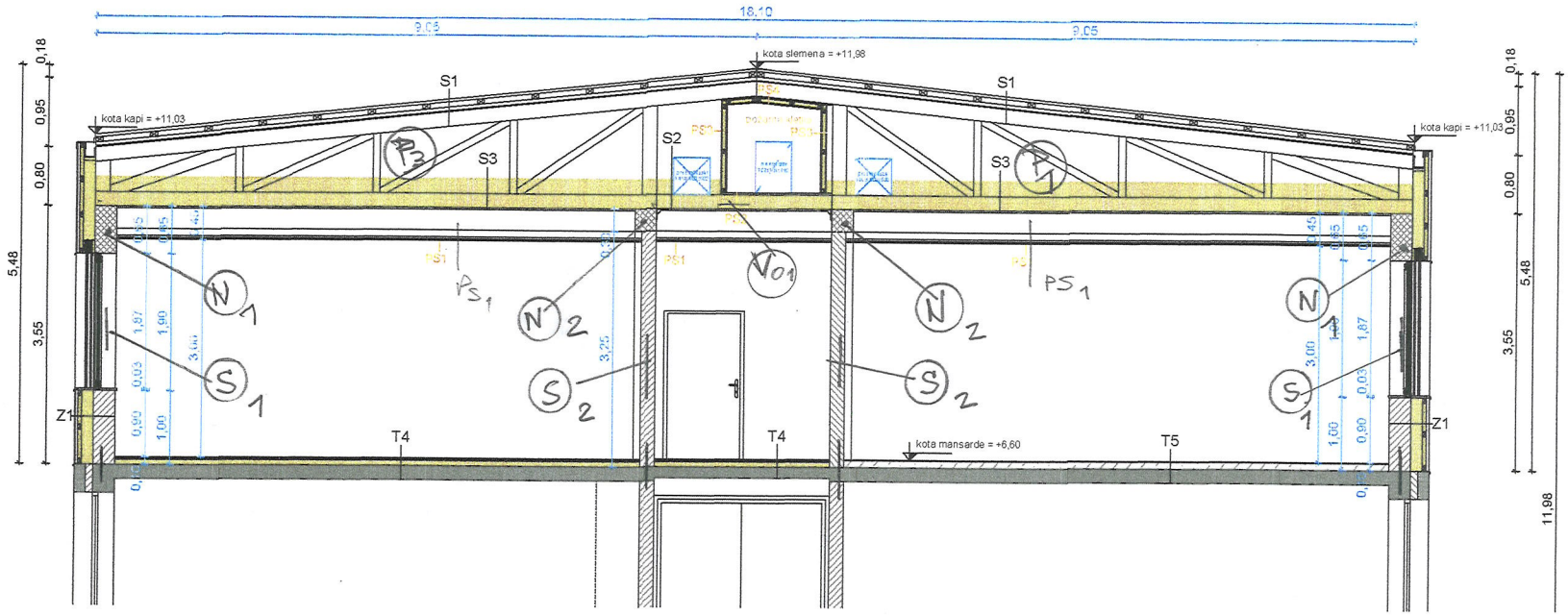
1:100



A2

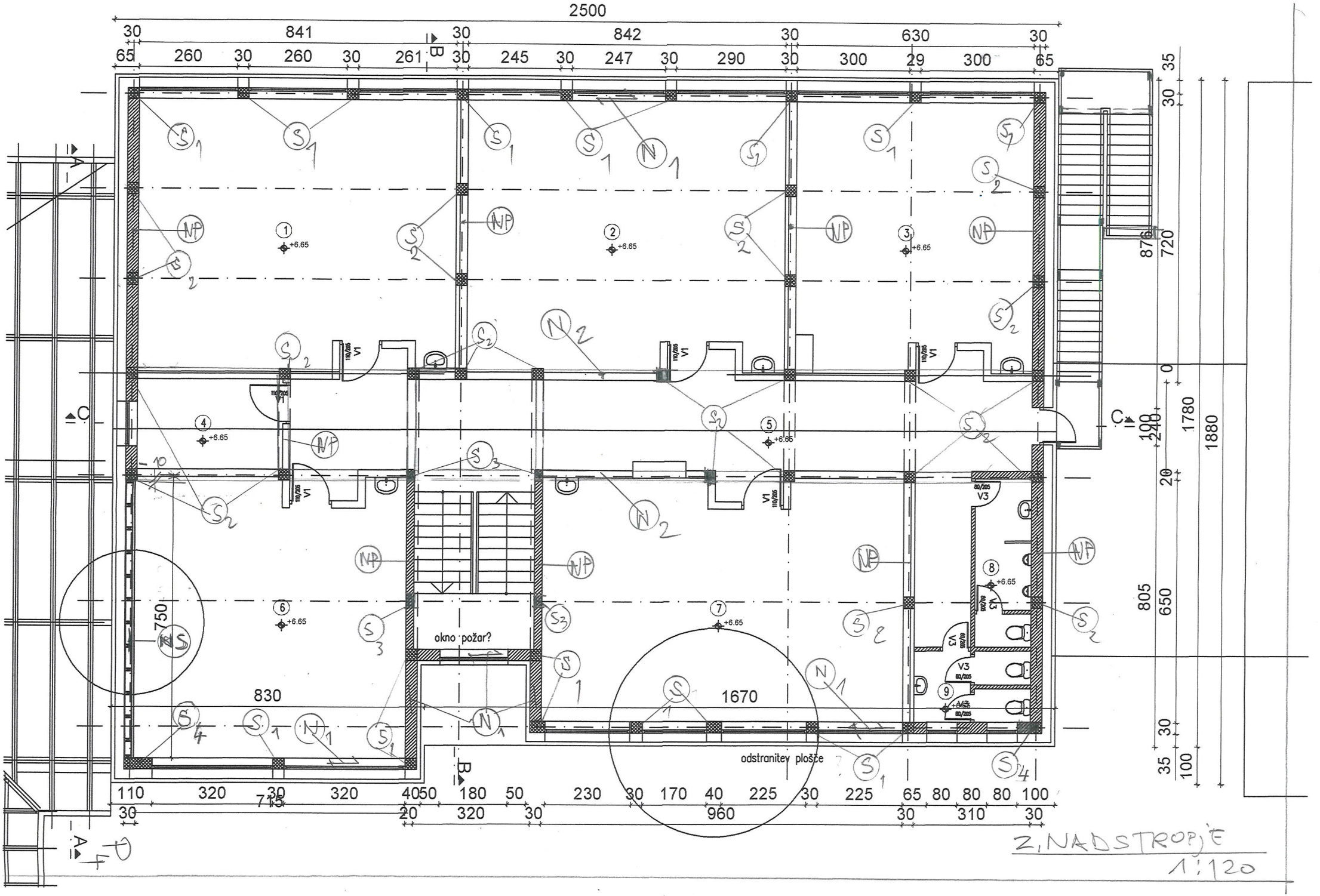
PREREZ 1:100

1:100



PRZEKRÓJ 1:100

1:120



2. NADSTROPJE

1:120

STATIKA

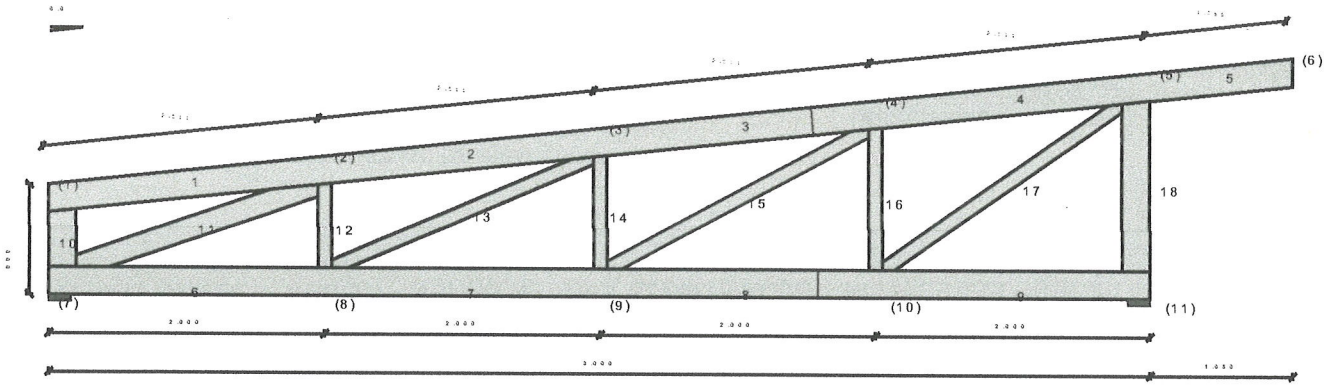
STATICNI IZRACUN

OSN. VREDN. GEOMETR.
razpon : 8000 mm; zac. visina: 800 mm; naklon : 6.0 sto ;
razmak letev : 800 mm

OSN. VREDNOSTI OBTEZB
normir.obtezba N/m2: +VETER
normir.obtezba N/m2:
razmak n: 1200 mm; LT,kritina: 500 N/m22; obtezba snega : 1500 N/m22
LT nosilca: 200 N/m22; Sp.pas : 400 N/m22+200 N/m22
pritisk vetra q: +500 [N/m2]

DIMENZ. PASNIC
debelina l.: 60 mm (1 krat)
zg. pas : 200; sp. pas : 200;
diagonale: 200; 160; 100; 100; 100; 100; 100;
100; 200;

SISTEM.skica

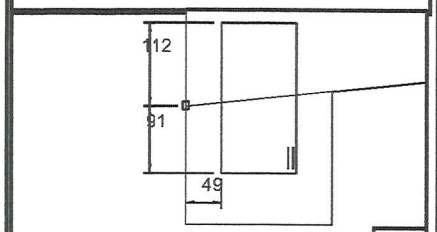


SPOROCILO
dimenzionirano: EN 1995-1-1 (EuroCode5) EC5
dolocitev ukl. dolzi.: po DIN 1052.
Obt. krovca upostevana povsod spodnjipasovi
Pas je pridrzan na 800 mm (po posevnini).

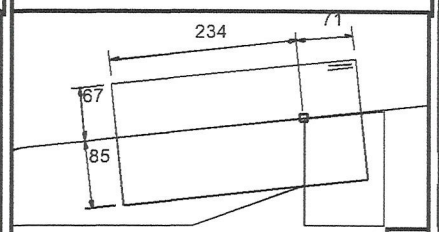
NAPETOST OK DEFORMACIJE OK; JEZEVKE OK

25

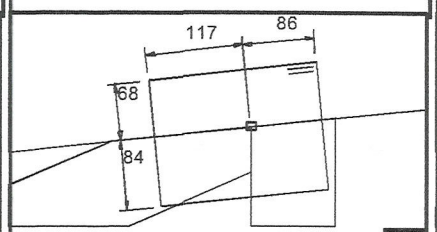
1.WOLF15N 102/203 X=-112 Y=49



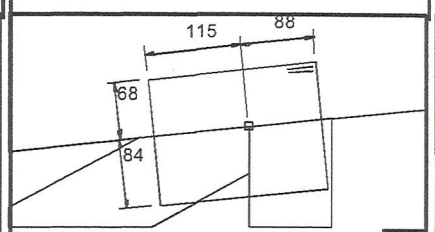
2.WOLF15N 152/305 X=-71 Y=67



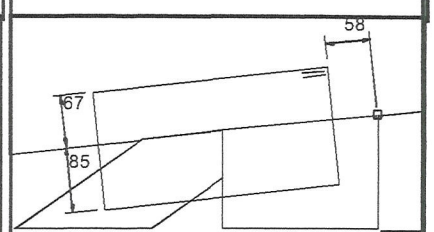
3.WOLF15N 152/203 X=-86 Y=68



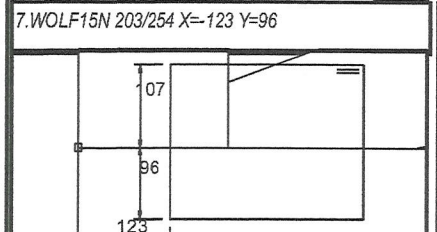
4.WOLF15N 152/203 X=-88 Y=68



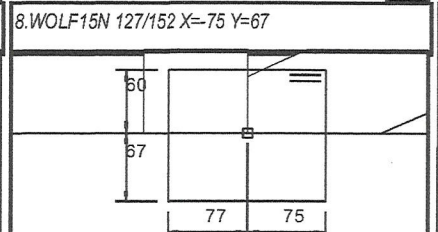
5.WOLF15N 152/305 X=58 Y=67



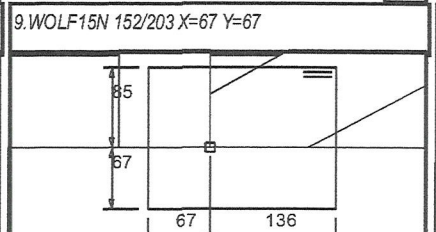
7.WOLF15N 203/254 X=-123 Y=96



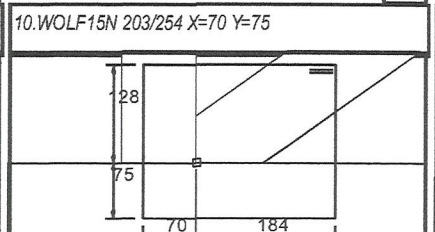
8.WOLF15N 127/152 X=-75 Y=67



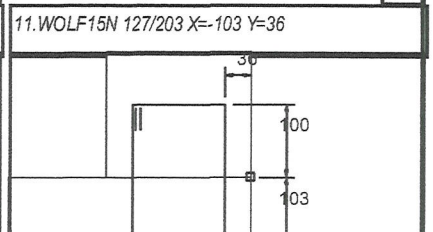
9.WOLF15N 152/203 X=67 Y=67



10.WOLF15N 203/254 X=70 Y=75



11.WOLF15N 127/203 X=-103 Y=36

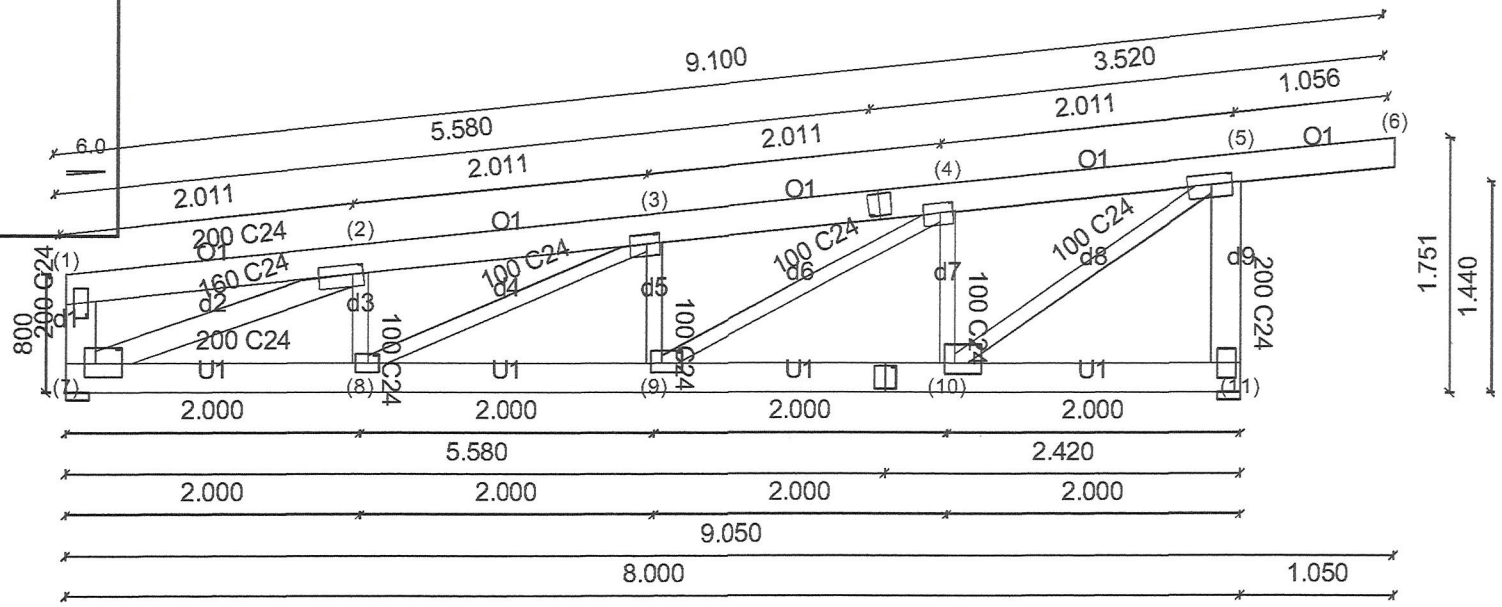


STIKI :

pas 1 WOLF15N 152/152 pri5580

pas 2 WOLF15N 152/152 pri5580

Teza nosilca :159.0kg

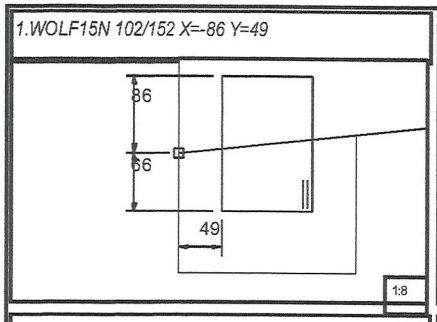


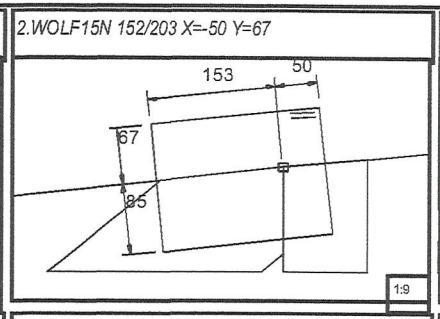
P1

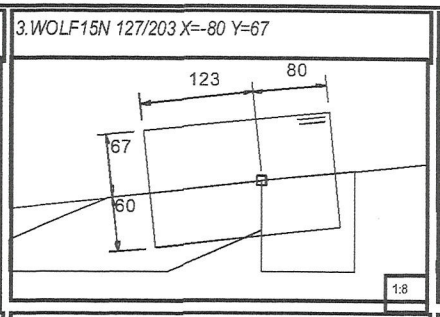
WOLFTRUSS8.91	ST. NOSILCEV: 7	OBTEZBE: 500 N/m2)	,RAZMAK LET.: 800mm	STURM D.O.O., GOMILSKO 30 A, 3303 GOMILSKO	
	ST. VECKRAT : 1			OZN.NOSIL P1 7kosov	25.9.2025
	DEBEL.: 60	IMPREGNACIJA Keines	NAROCILO: os	PROJEKT: OS Tabor	
	RAZMAK : 1200mm			MERILO 1:50	IZVEDBA :
				INVESTIT. : os Tabor	

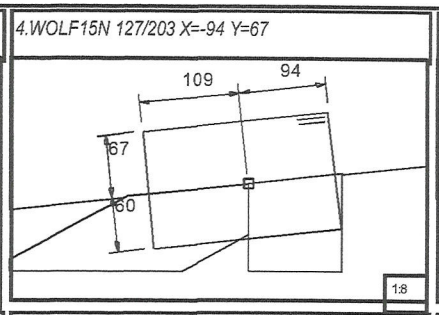
1. PRED IZDELAVO NOSILCEV MORA POOBLAŠČENA OSEBA IZDELOVALCA
2. KONTROLIRATI PODATKE UPOŠTEVANE V STAT. RAČUNU (MERE, NAKLON, ...) !
3. PRI IZDELAVI IN MONTAŽI UPOŠTEVATI NAVODILA ZA MONTAŽO !
4. IZRAČUN PO EC5 (DIN 1052)

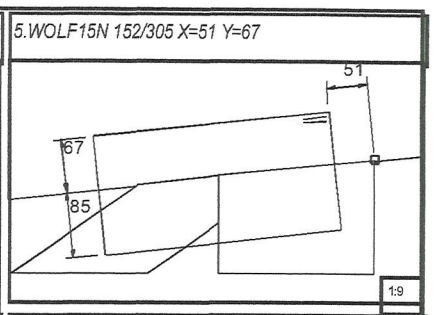
wolf SYSTEM

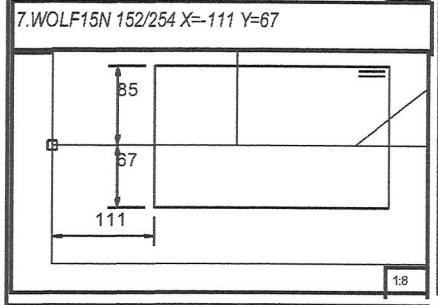


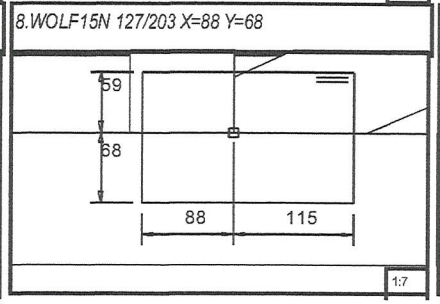


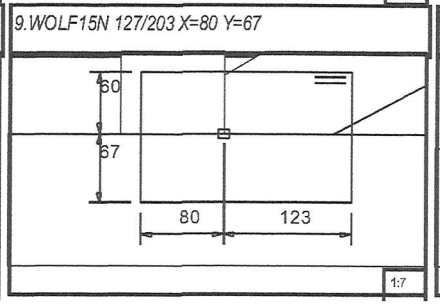


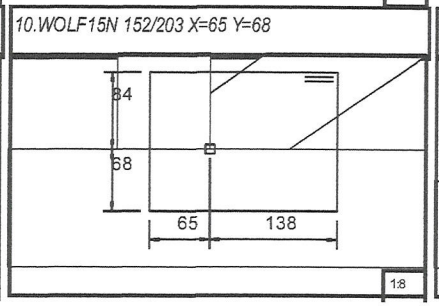


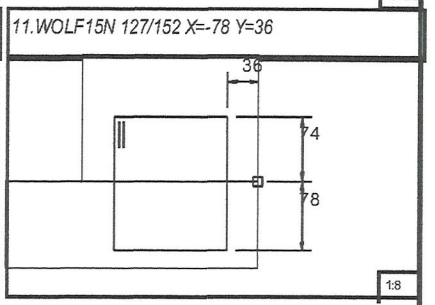










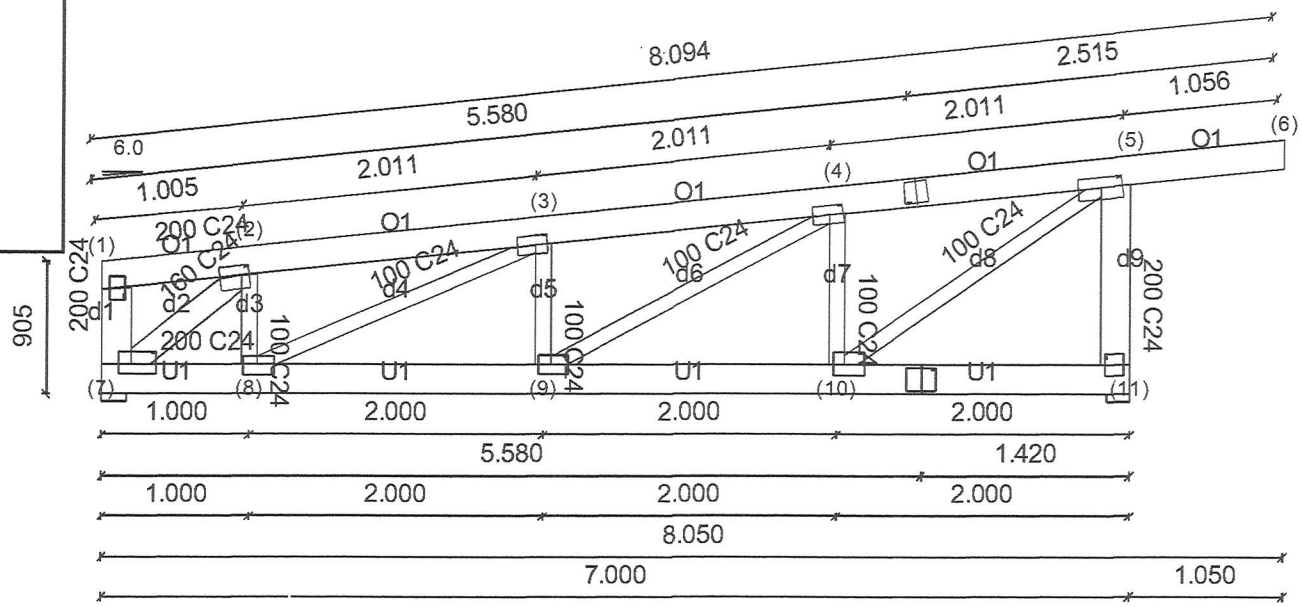


STIKI :

pas 1 WOLF15N 152/152 pri5580

pas 2 WOLF15N 152/203 pri5580

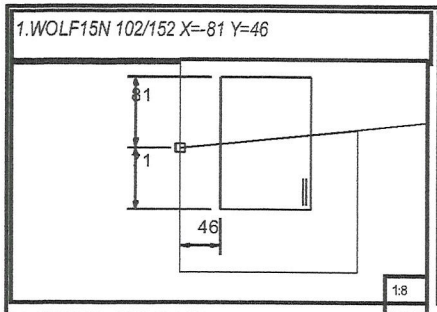
Teza nosilca :141.7kg

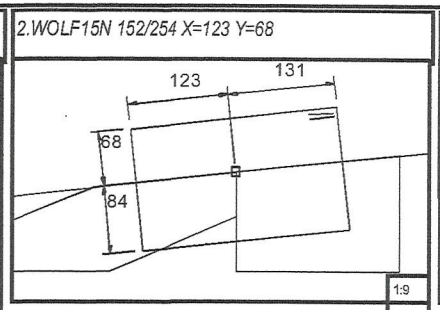


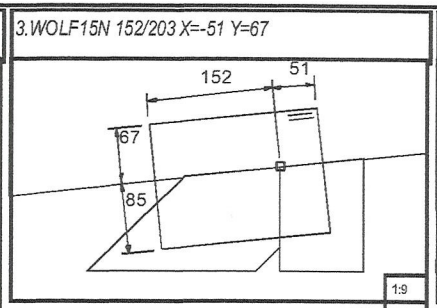
P2

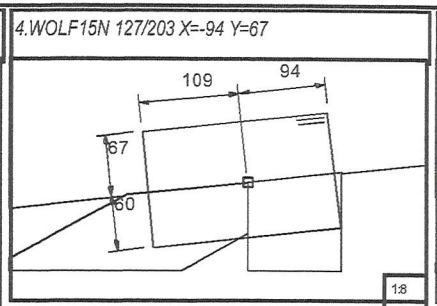
WOLFTRUSS8.91	ST. NOSILCEV: 12 ST. VECKRAT : 1 DEBEL.: 60 RAZMAK : 1200mm	OBTEZBE: 500 N/m2) IMPREGNACIJA keine	,RAZMAK LET.: 800mm NAROCILO: os	STURM D.O.O., GOMILSKO 30 A, 3303 GOMILSKO OZN.NOSIL P2 12kosov 26.9.2025 PROJEKT: OS Tabor MERILO 1:50 IZVEDBA : INVESTIT. : os Tabor	wolf SYSTEM

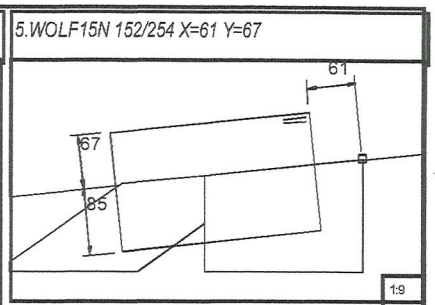
1. PRED IZDELAVO NOSILCEV MORA POOBLAŠČENA OSEBA IZDELOVALCA
2. KONTROLIRATI PODATKE UPOŠTEVANE V STAT. RAČUNU (MERE, NAKLON, ...) !
3. PRI IZDELAVI IN MONTAŽI UPOŠTEVATI NAVODILA ZA MONTAŽO !
4. IZRAČUN PO EC5 (DIN 1052)

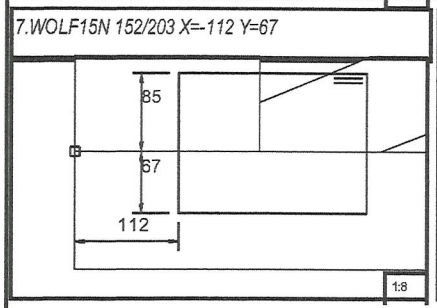


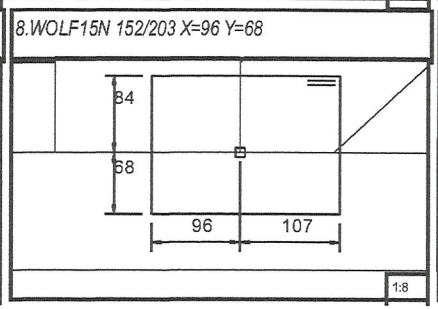


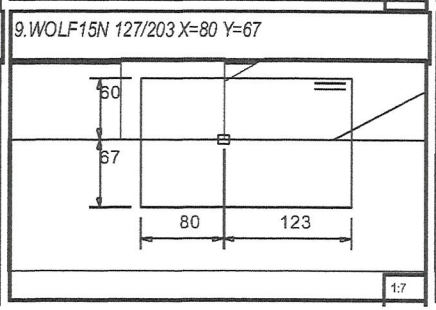


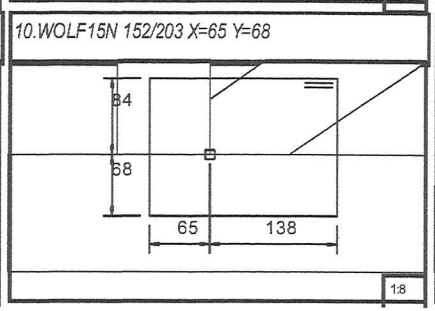


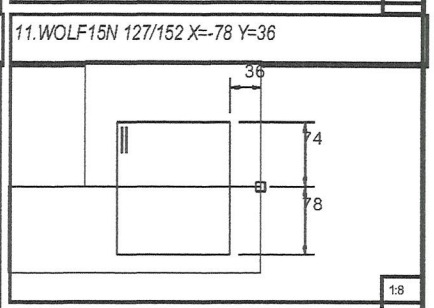










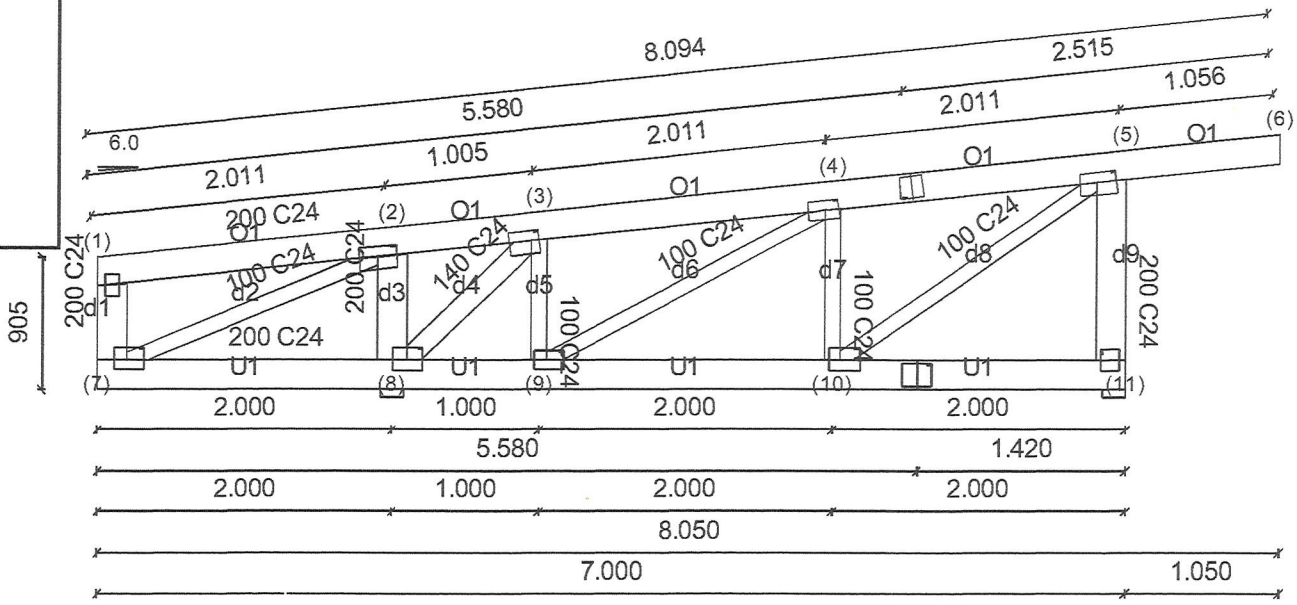


STIKI :

pas 1 WOLF15N 152/152 pri5580

pas 2 WOLF15N 152/203 pri5580

Teza nosilca :144.0kg

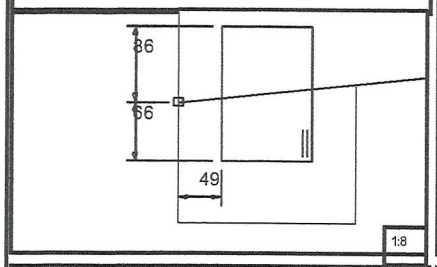


P2.2

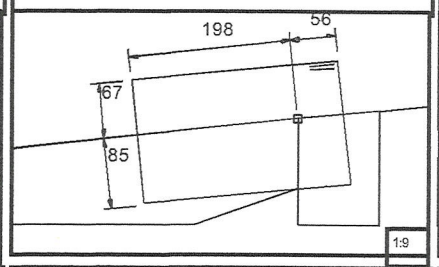
WOLFTRUSS8.91	ST. NOSILCEV: 2 ST. VECKRAT : 1 DEBEL.: 60 RAZMAK : 1200mm	OBTEZBE: 500 N/m2) IMPREGNACIJA keine	,RAZMAK LET.: 1200mm NAROCILO: os	STURM D.O.O., GOMILSKO 30 A, 3303 GOMILSKO OZN.NOSIL P2.2 2kosov 26.9.2025 PROJEKT: OS Tabor MERILO 1:50 IZVEDBA : INVESTIT. : os Tabor	WOLF SYSTEM

1. PRED IZDELAVO NOSILCEV MORA POOBLAŠČENA OSEBA IZDELOVALCA
2. KONTROLIRATI PODATKE UPOŠTEVANE V STAT. RAČUNU (MERE, NAKLON, ...) !
3. PRI IZDELAVI IN MONTAŽI UPOŠTEVATI NAVODILA ZA MONTAŽO !
4. IZRAČUN PO EC5 (DIN 1052)

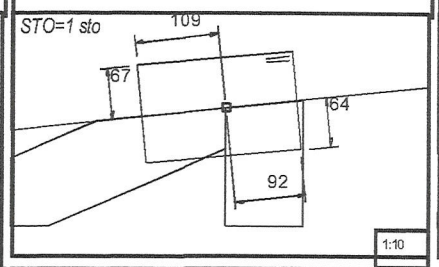
1.WOLF15N 102/152 X=-86 Y=49



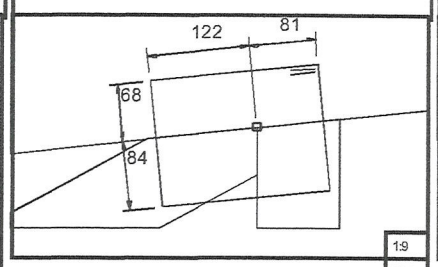
2.WOLF15N 152/254 X=-56 Y=67



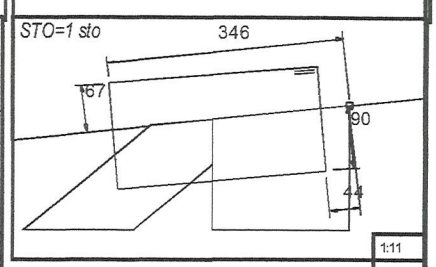
3.WOLF15N 127/203 X=109 Y=67



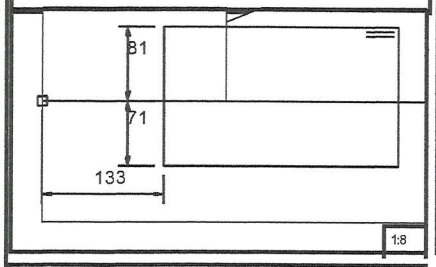
4.WOLF15N 152/203 X=-81 Y=68



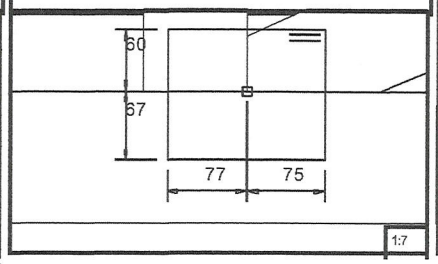
5.WOLF15N 152/305 X=346 Y=67



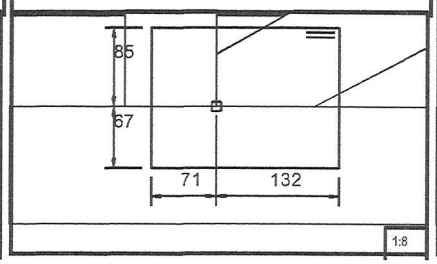
7.WOLF15N 152/254 X=-133 Y=71



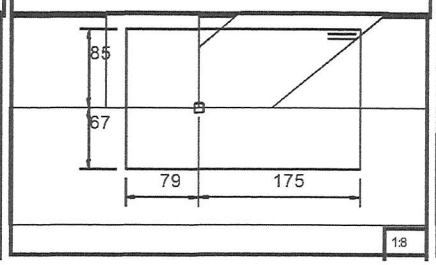
8.WOLF15N 127/152 X=-75 Y=67



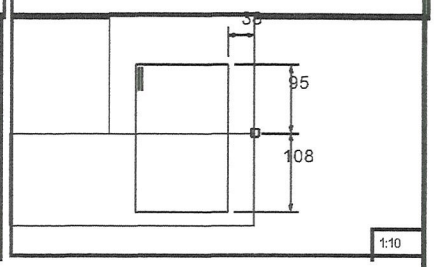
9.WOLF15N 152/203 X=71 Y=67



10.WOLF15N 152/254 X=79 Y=67



11.WOLF15N 127/203 X=-108 Y=38

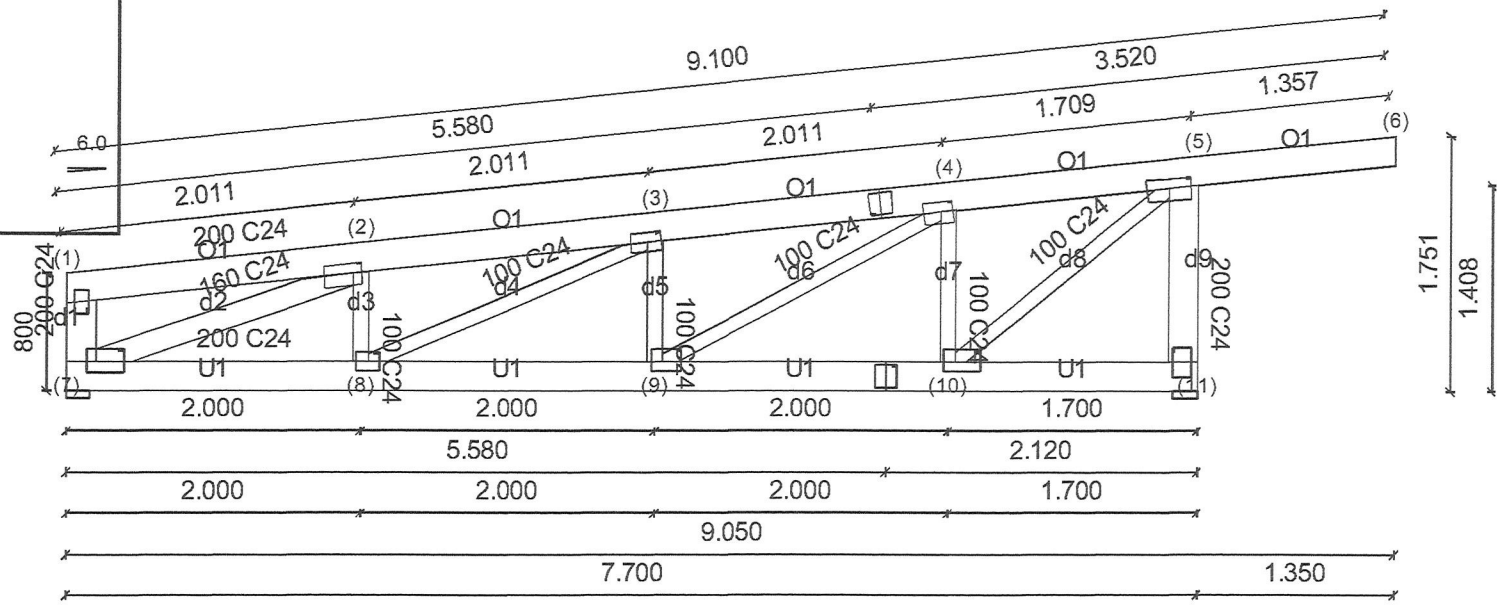


STIKI :

pas 1 WOLF15N 152/152 pri5580

pas 2 WOLF15N 152/152 pri5580

Teza nosilca :154.8kg

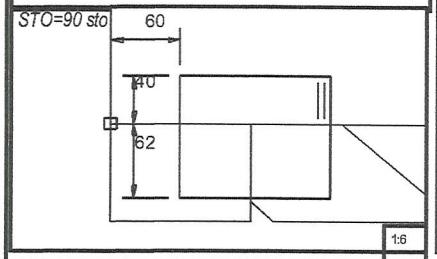


P3

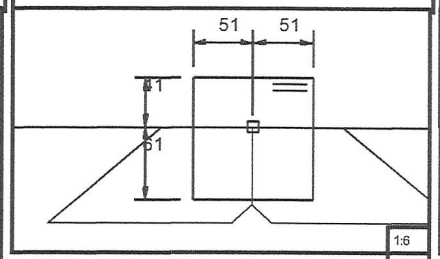
WOLFTRUSS8.91	ST. NOSILCEV: 21	OBTEZBE: 500 N/m2)	,RAZMAK LET.: 800mm	STURM D.O.O., GOMILSKO 30 A, 3303 GOMILSKO	
	ST. VECKRAT : 1			OZN.NOSIL P3 2lkosov	25.9.2025
	DEBEL.: 60	IMPREGNACIJA Keines	NAROCILO: os	PROJEKT: OS Tabor	
	RAZMAK : 1200mm			MERILO 1:50	IZVEDBA :
				INVESTIT. : os Tabor	

1. PRED IZDELAVO NOSILCEV MORA POOBLAŠČENA OSEBA IZDELOVALCA
2. KONTROLIRATI PODATKE UPOŠTEVANE V STAT. RAČUNU (MERE, NAKLON, ...) !
3. PRI IZDELAVI IN MONTAŽI UPOŠTEVATI NAVODILA ZA MONTAŽO !
4. IZRAČUN PO EC5 (DIN 1052)

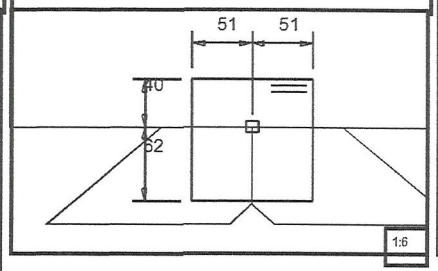
1+5.WOLF15N 127/102 X=-60 Y=40



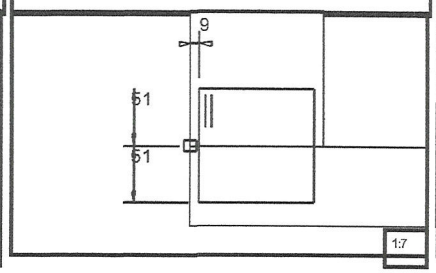
2+4.WOLF15N 102/102 X=51 Y=41



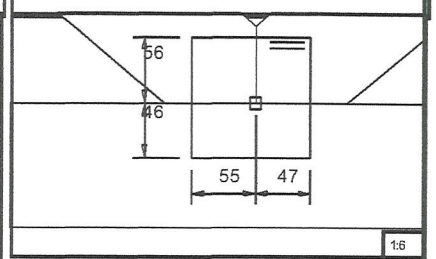
3.WOLF15N 102/102 X=51 Y=40



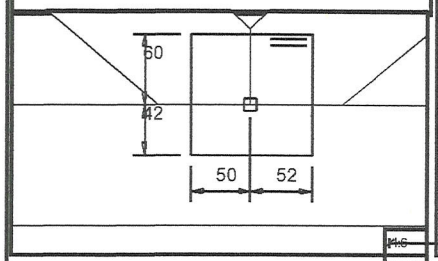
6+11.WOLF15N 102/102 X=-51 Y=9



7+10.WOLF15N 102/102 X=-47 Y=46



8+9.WOLF15N 102/102 X=50 Y=42

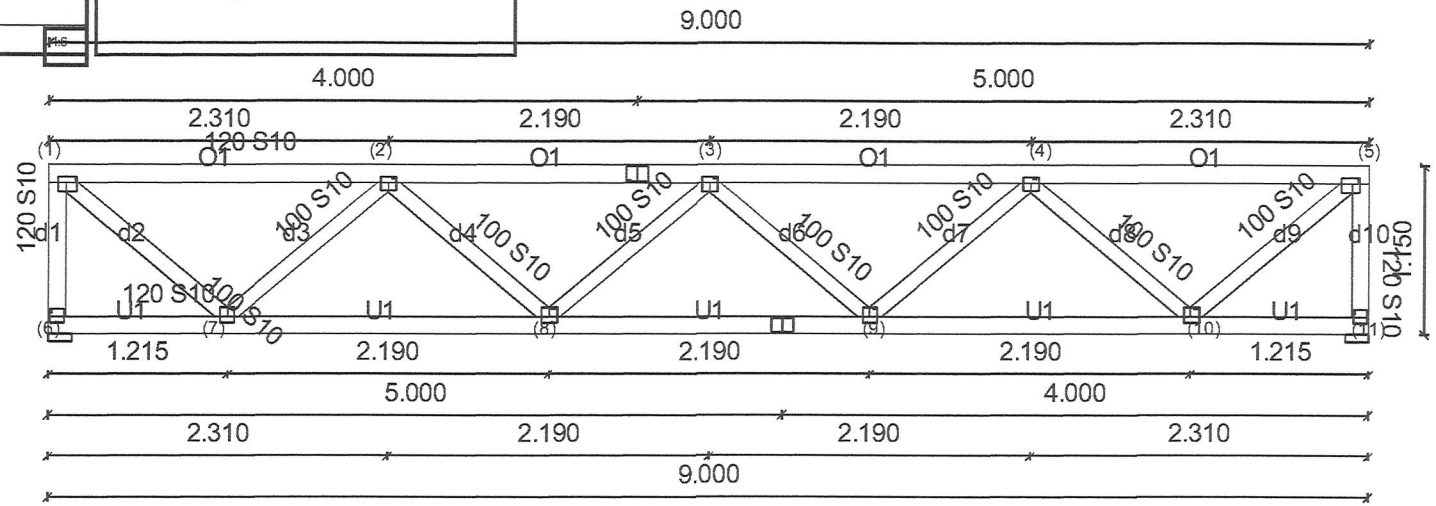


STIKI :

pas 1 WOLF15N 102/152 pri4000

pas 2 WOLF15N 102/152 pri5000

Teza nosilca :109.0kg



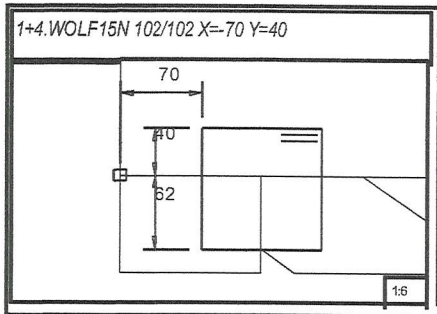
MEJA ZG, PASOVI!

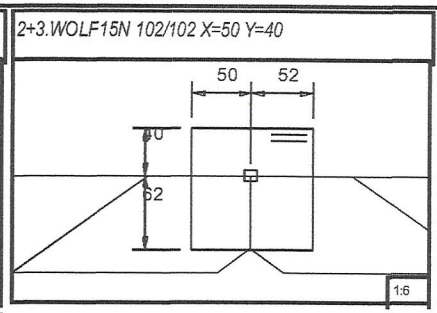
Z1

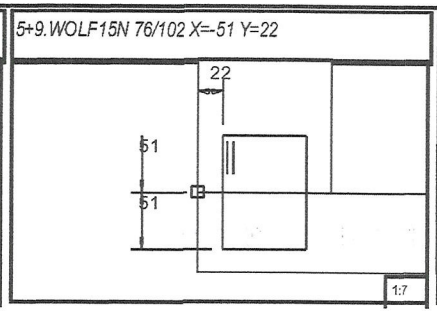
Vročiti: naslov
Vložiti: v spis

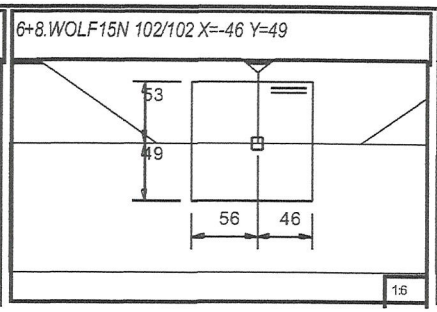
WOLFTRUSS8.91	ST. NOSILCEV: 6 ST. VECKRAT : 1 DEBEL.: 60 RAZMAK : 1000mm	OBTEZBE: posebne obt. IMPREGNACIJA Keines	,RAZMAK LET.: 500mm NAROCILO: os	STURM D.O.O., GOMILSKO 30 A, 3303 GOMILSKO OZN.NOSIL Z1 6kosov 25.9.2025 PROJEKT: OS Tabor MERILO 1:50 IZVEDBA : INVESTIT. : os Tabor	WOLF SYSTEM

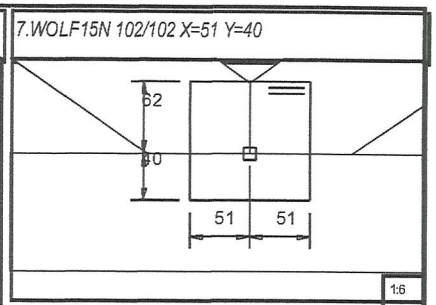
1. PRED IZDELAVO NOSILCEV MORA POOBLAŠČENA OSEBA IZDELOVALCA
2. KONTROLIRATI PODATKE UPOŠTEVANE V STAT. RAČUNU (MERE, NAKLON, ...) !
3. PRI IZDELAVI IN MONTAŽI UPOŠTEVATI NAVODILA ZA MONTAŽO !
4. IZRAČUN PO EC5 (DIN 1052)









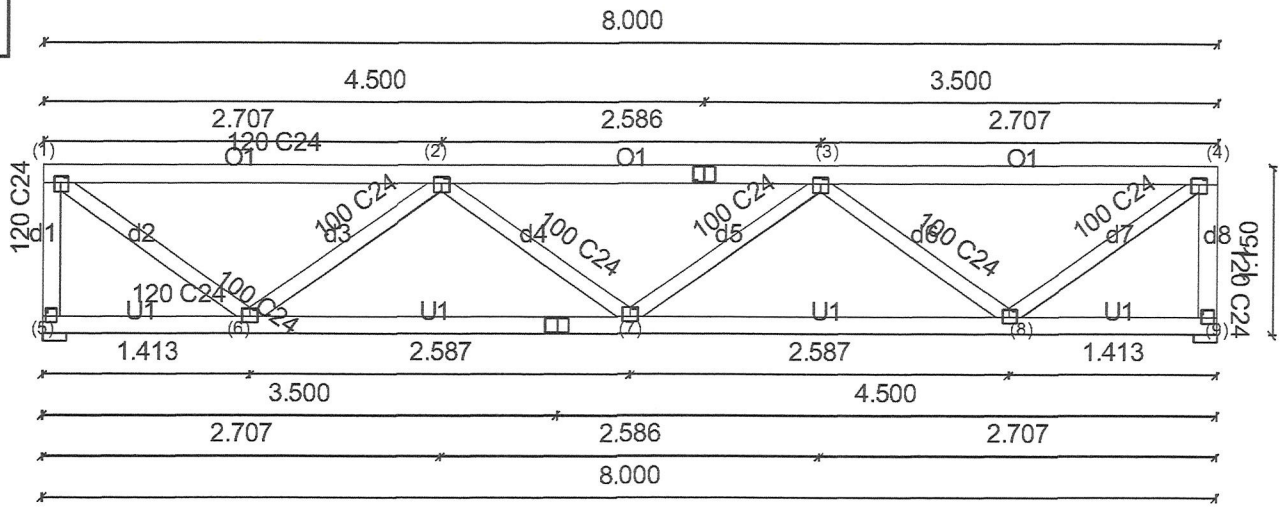


STIKI :

pas 1 WOLF15N 102/152 pri4500

pas 2 WOLF15N 102/152 pri3500

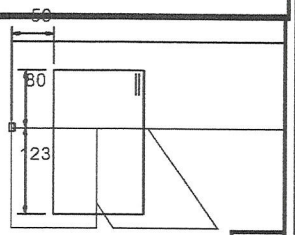
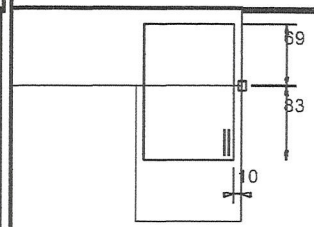
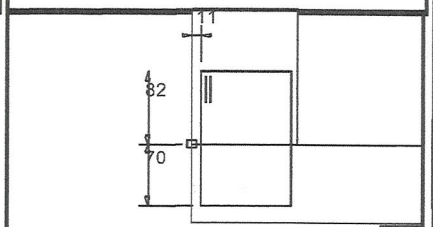
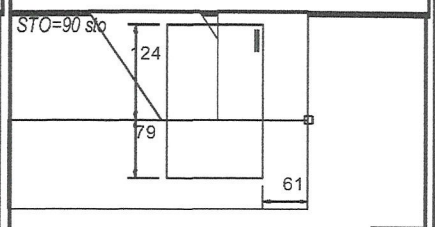
Teza nosilca :95.4kg

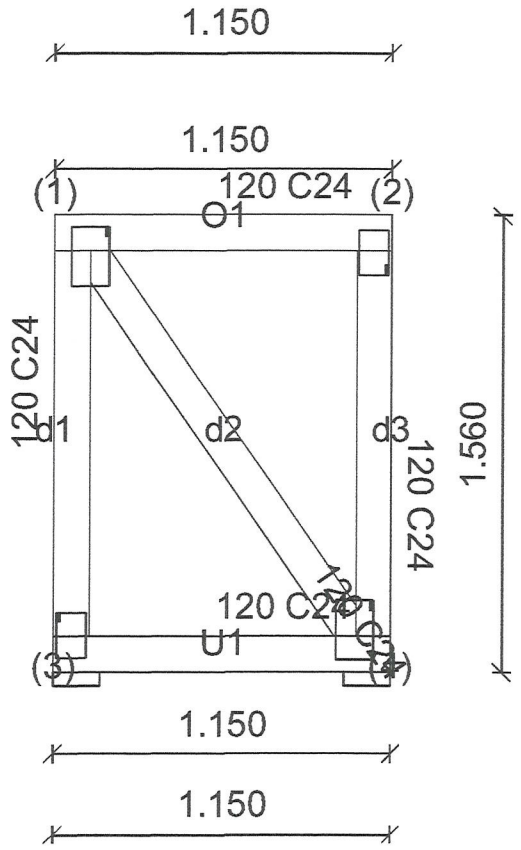


Z2

WOLFTRUSS8.91	ST. NOSILCEV: 3 ST. VECKRAT : 1 DEBEL.: 60 RAZMAK : 1000mm	OBTEZBE: posebne obt. IMPREGNACIJA: Keines	,RAZMAK LET.: 500mm NAROCILO: os	STURM D.O.O., GOMILSKO 30 A, 3303 GOMILSKO OZN.NOSIL Z2 3kosov 25.9.2025 PROJEKT: OS Tabor MERILO 1:50 IZVEDBA : INVESTIT. : os Tabor	WOLF SYSTEM

1. PRED IZDELAVO NOSILCEV MORA POOBLAŠČENA OSEBA IZDELOVALCA
2. KONTROLIRATI PODATKE UPOŠTEVANE V STAT. RAČUNU (MERE, NAKLON, ...) !
3. PRI IZDELAVI IN MONTAŽI UPOŠTEVATI NAVODILA ZA MONTAŽO !
4. IZRAČUN PO EC5 (DIN 1052)

1.WOLF15N 127/203 X=-59 Y=80	2.WOLF15N 102/152 X=-69 Y=10	3.WOLF15N 102/152 X=-70 Y=11	4.WOLF15N 127/203 X=61 Y=79	STIKI :
STO=90 sto			STO=90 sto	
				Teza nosilca :25.9kg

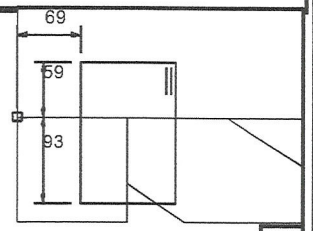
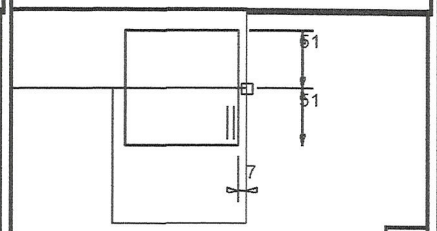
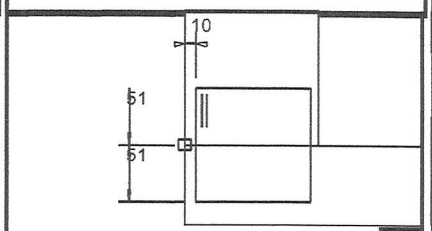
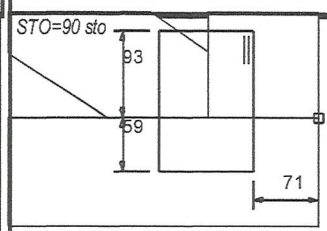


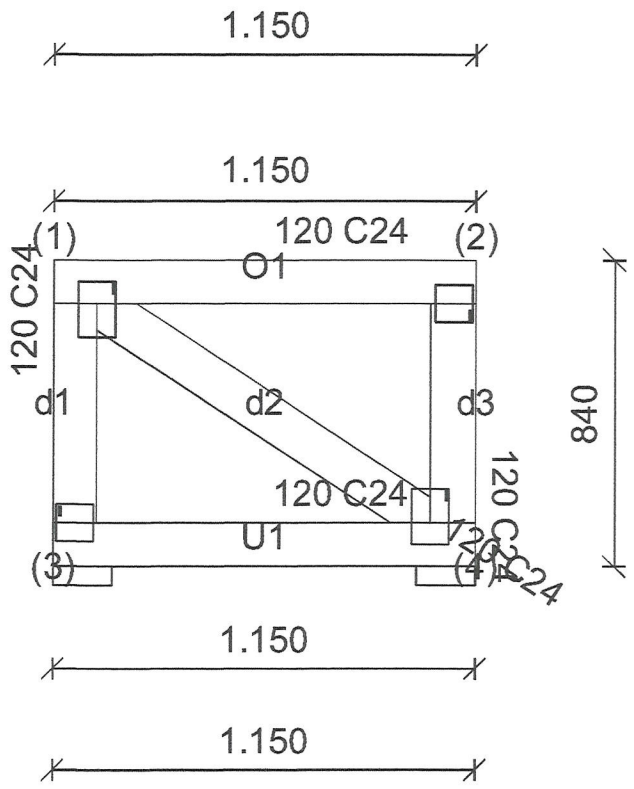
NA LEŽIŠČIH!

Z3

WOLFTRUSS8.91	ST. NOSILCEV: 9 ST. VECKRAT : 1 DEBEL.: 60 RAZMAK : 100mm	OBTEŽBE: posebne obt. IMPREGNACIJA: Keines	,RAZMAK LET.: 800mm NAROČILO: os	STURM D.O.O., GOMILSKO 30 A, 3303 GOMILSKO OZN.NOSIL Z3 9kosov 25.9.2025 PROJEKT: OS Tabor MERILO 1:25 IZVEDBA : INVESTIT. : os Tabor	wolf SYSTEM

1. PRED IZDELAVO NOSILCEV MORA POOBLAŠČENA OSEBA IZDELOVALCA
2. KONTROLIRATI PODATKE UPOŠTEVANE V STAT. RAČUNU (MERE, NAKLON, ...) !
3. PRI IZDELAVI IN MONTAŽI UPOŠTEVATI NAVODILA ZA MONTAŽO !
4. IZRAČUN PO EC5 (DIN 1052)

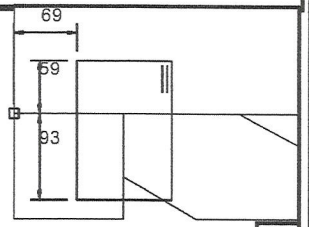
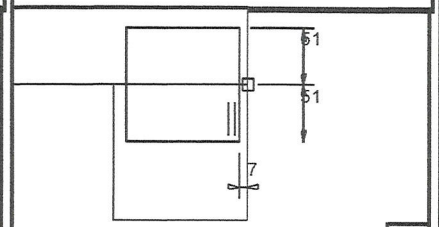
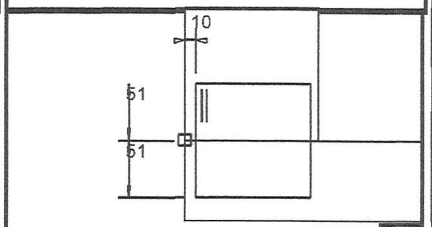
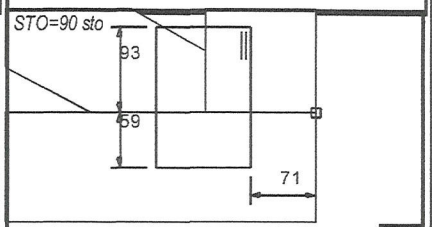
1.WOLF15N 102/152 X=-69 Y=59	2.WOLF15N 102/102 X=-51 Y=7	3.WOLF15N 102/102 X=-51 Y=10	4.WOLF15N 102/152 X=71 Y=59	STIKI :
STO=90 sto 			STO=90 sto 	Teza nosilca :17.9kg

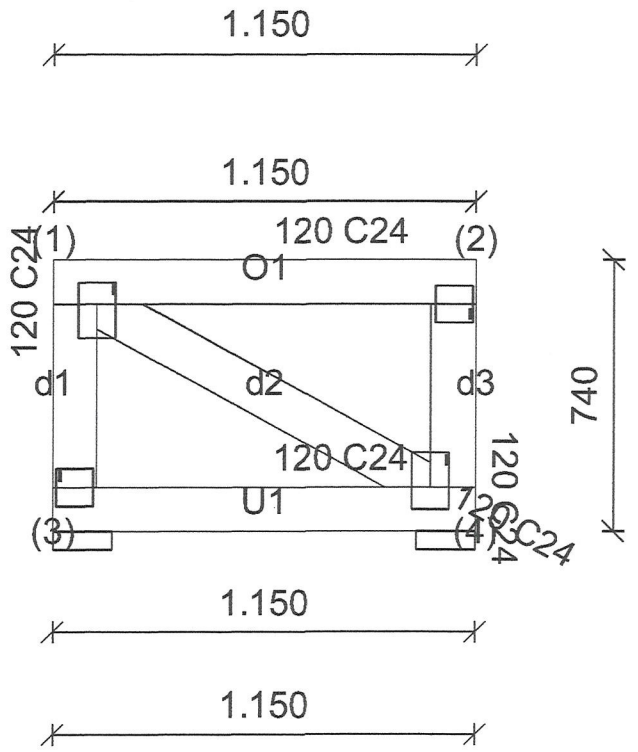


Z4

WOLFTRUSS8.91	ST. NOSILCEV: 3 ST. VECKRAT : 1 DEBEL.: 60 RAZMAK : 100mm	OBTEŽBE: posebne obt. IMPREGNACIJA Keines	,RAZMAK LET.: 800mm NAROČILO: os	STURM D.O.O., GOMILSKO 30 A, 3303 GOMILSKO OZN.NOSIL Z4 3kosov 25.9.2025 PROJEKT: OS Tabor MERILO 1:20 IZVEDBA : INVESTIT. : os Tabor	wolf SYSTEM

1. PRED IZDELAVO NOSILCEV MORA POOBLAŠČENA OSEBA IZDELOVALCA
2. KONTROLIRATI PODATKE UPOŠTEVANE V STAT. RAČUNU (MERE, NAKLON, ...) !
3. PRI IZDELAVI IN MONTAŽI UPOŠTEVATI NAVODILA ZA MONTAŽO !
4. IZRAČUN PO EC5 (DIN 1052)

1.WOLF15N 102/152 X=-69 Y=59	2.WOLF15N 102/102 X=-51 Y=7	3.WOLF15N 102/102 X=-51 Y=10	4.WOLF15N 102/152 X=71 Y=59	STIKI :
STO=90 sto			STO=90 sto	
				Teza nosilca :17.0kg

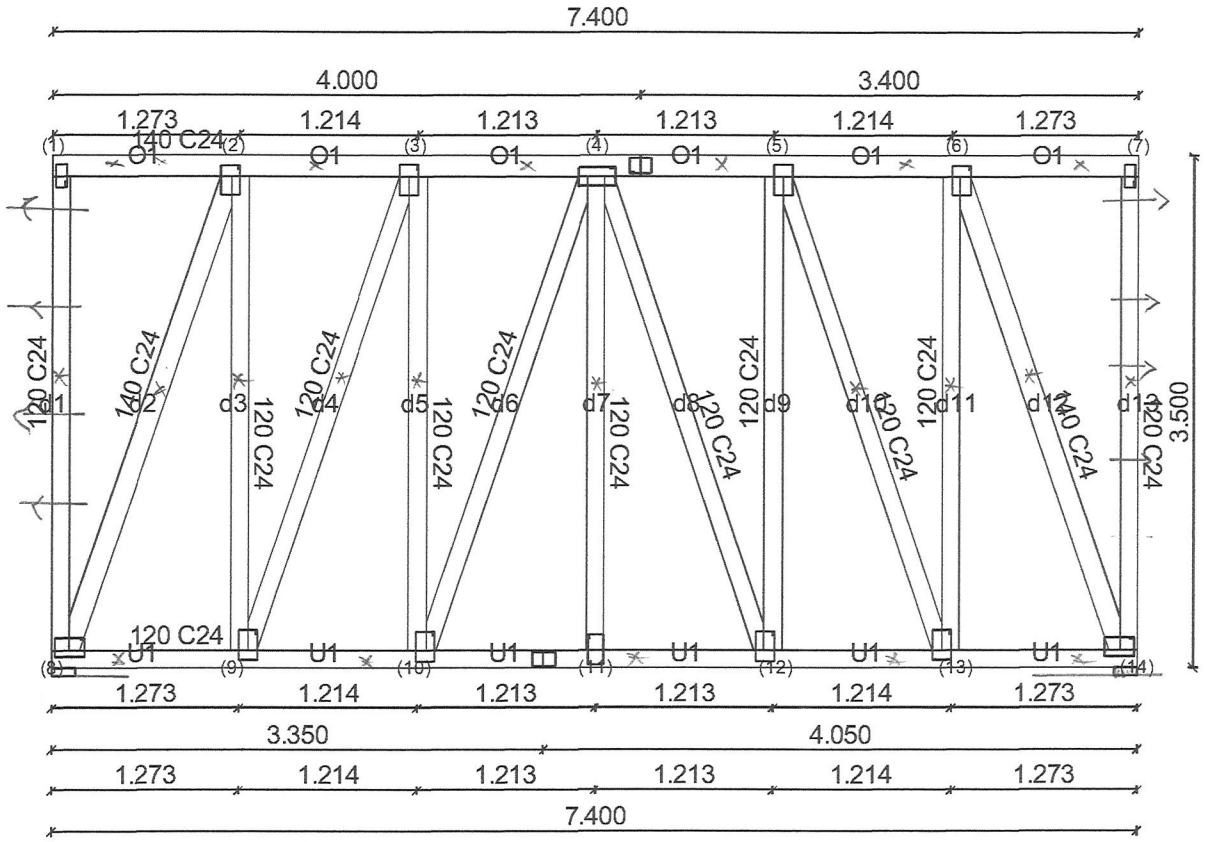


Z5

WOLFTRUSS8.91	ST. NOSILCEV: 6 ST. VECKRAT : 1 DEBEL.: 60 RAZMAK : 100mm	OBTEZBE: posebne obt. IMPREGNACIJA Keines	,RAZMAK LET.: 800mm NAROCILO: os	STURM D.O.O., GOMILSKO 30 A, 3303 GOMILSKO OZN.NOSIL Z5 6kosov 25.9.2025 PROJEKT: OS Tabor MERILO 1:20 IZVEDBA : INVESTIT. : os Tabor

1. PRED IZDELAVO NOSILCEV MORA POOBLAŠČENA OSEBA IZDELOVALCA
2. KONTROLIRATI PODATKE UPOŠTEVANE V STAT. RAČUNU (MERE, NAKLON, ...) !
3. PRI IZDELAVI IN MONTAŽI UPOŠTEVATI NAVODILA ZA MONTAŽO !
4. IZRAČUN PO EC5 (DIN 1052)

WOLF SYSTEM



* zmotnjeno!
($e_m \sim 7^3 m^3$)

sidrano v VV! DZ. STEBRE

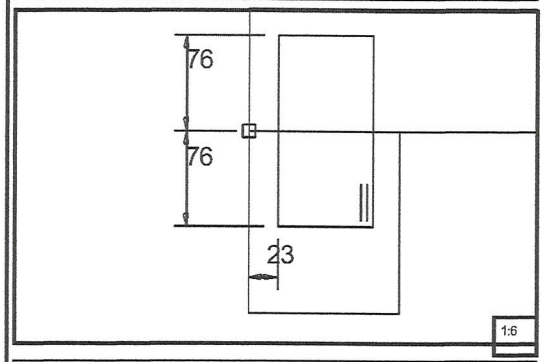
NS

WOLFTRUSS8.91	ST. NOSILCEV: 1N*	OBTEŽBE: 500 N/m2)	RAZMAK LET.: 1250mm	STURM d.o.o., Gomilsko 30 a, 3303 Gomilsko	
	ST. VEČKRAT: 3			OZN.NOSIL NS 1x3fach	25.9.2025
	DEBEL.: 60	IMPREGNACIJA Keines	NAROČILO: os	PROJEKT: OS Tabor	
	RAZMAK: 2000mm			MERILO 1:50	IZVEDBA:
				INVESTIT.: os Tabor	

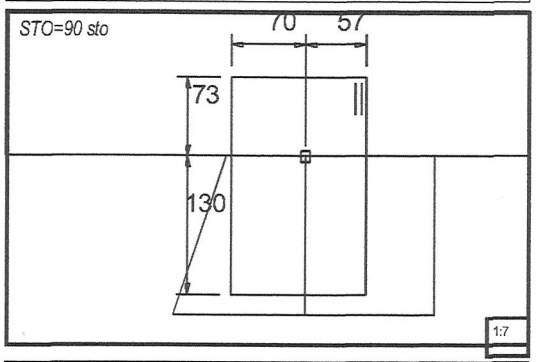
1. Pred izdelavo nosilcev mora pooblaščena oseba izdelovalca
2. kontrolirati podatke upoštevane v stat. računu (mere, naklon, ...) !
3. Pri izdelavi in montaži upoštevati navodila za montažo !
4. Izračun po EC5 (DIN 1052)

wolf SYSTEM

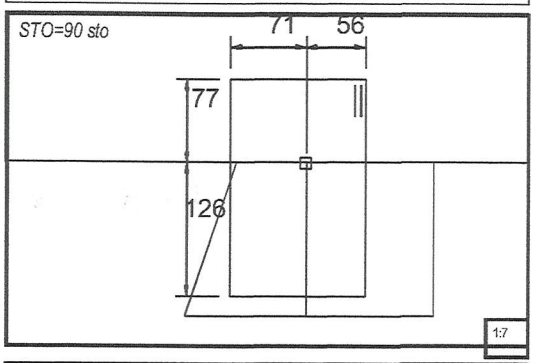
1+7.WOLF15N 76/152 X=-76 Y=23



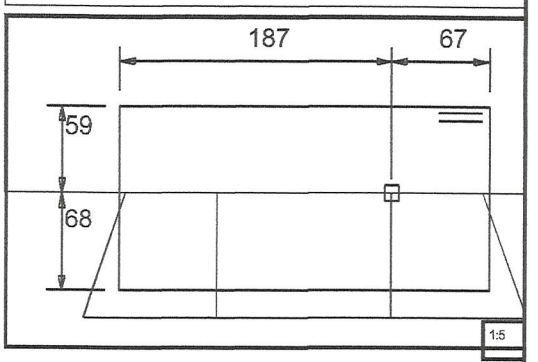
2+6.WOLF15N 127/203 X=-57 Y=73



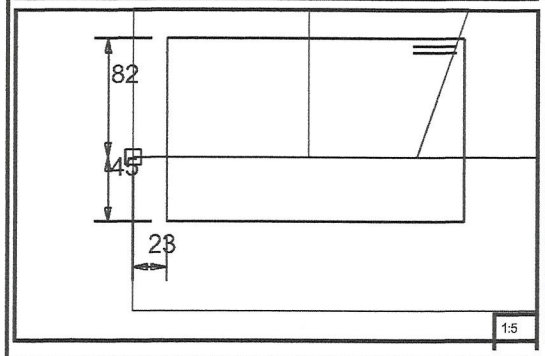
3+5.WOLF15N 127/203 X=-56 Y=77



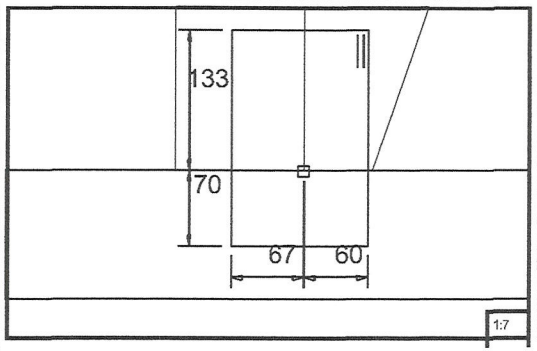
4.WOLF15N 127/254 X=-67 Y=59



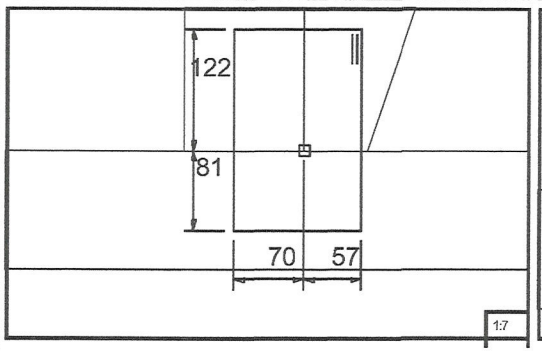
8+14.WOLF15N 127/203 X=-23 Y=45



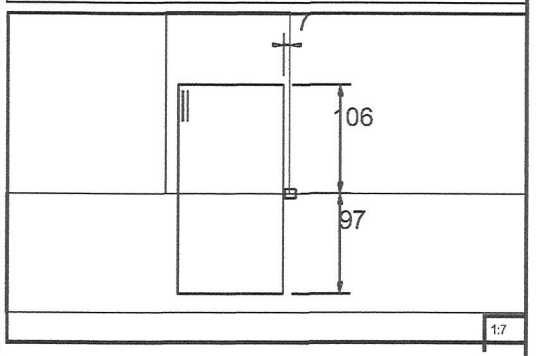
9+13.WOLF15N 127/203 X=-60 Y=70



10+12.WOLF15N 127/203 X=-57 Y=81



11.WOLF15N 102/203 X=-97 Y=7



STIKI :

pas 1 WOLF15N 102/152 pri4000

pas 2 WOLF15N 102/152 pri3350

Teza nosilca :226.3kg

sidrano v VV!

NS

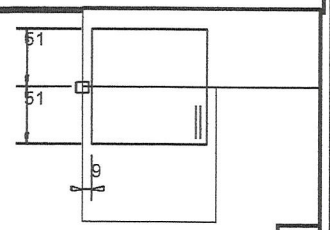
WOLFTRUSS8.91

ST. NOSILCEV:	1N*	OBTEŽBE:	,RAZMAK LET.:	STURM D.O.O., GOMILSKO 30 A, 3303 GOMILSKO	
ST. VECKRAT :	3	500 N/m2)	1250mm	OZN.NOSIL NS	1x3fach 25.9.2025
DEBEL.:	60	IMPREGNACIJA	NAROČILO:	PROJEKT:	OS Tabor
RAZMAK :	2000mm	Keines	os	MERILO 1:50	IZVEDBA :
				INVESTIT. :	os
					Tabor

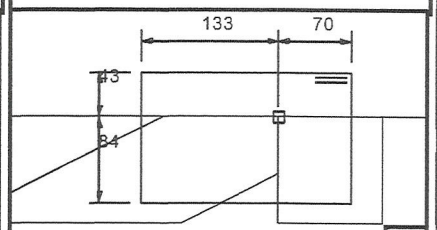
1. PRED IZDELAVO NOSILCEV MORA POOBLAŠČENA OSEBA IZDELOVALCA
2. KONTROLIRATI PODATKE UPOŠTEVANE V STAT. RAČUNU (MERE, NAKLON, ...) !
3. PRI IZDELAVI IN MONTAŽI UPOŠTEVATI NAVODILA ZA MONTAŽO !
4. IZRAČUN PO EC5 (DIN 1052)

wolf SYSTEM

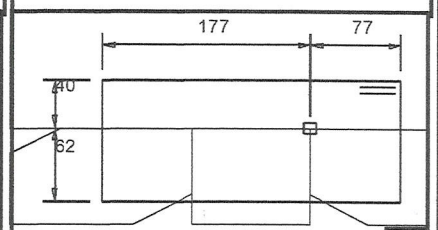
1+5.WOLF15N 102/102 X=-51 Y=9



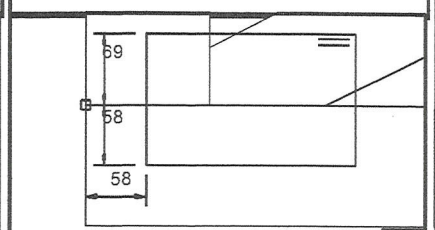
2+4.WOLF15N 127/203 X=-70 Y=43



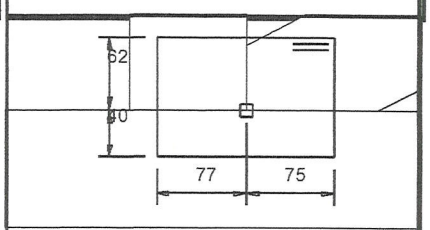
3.WOLF15N 102/254 X=-77 Y=40



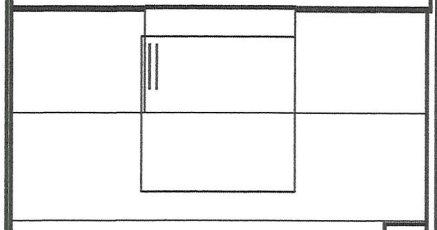
6+10.WOLF15N 127/203 X=-58 Y=58



7+9.WOLF15N 102/152 X=-75 Y=40



8.WOLF15N 102/102 X=-51 Y=0

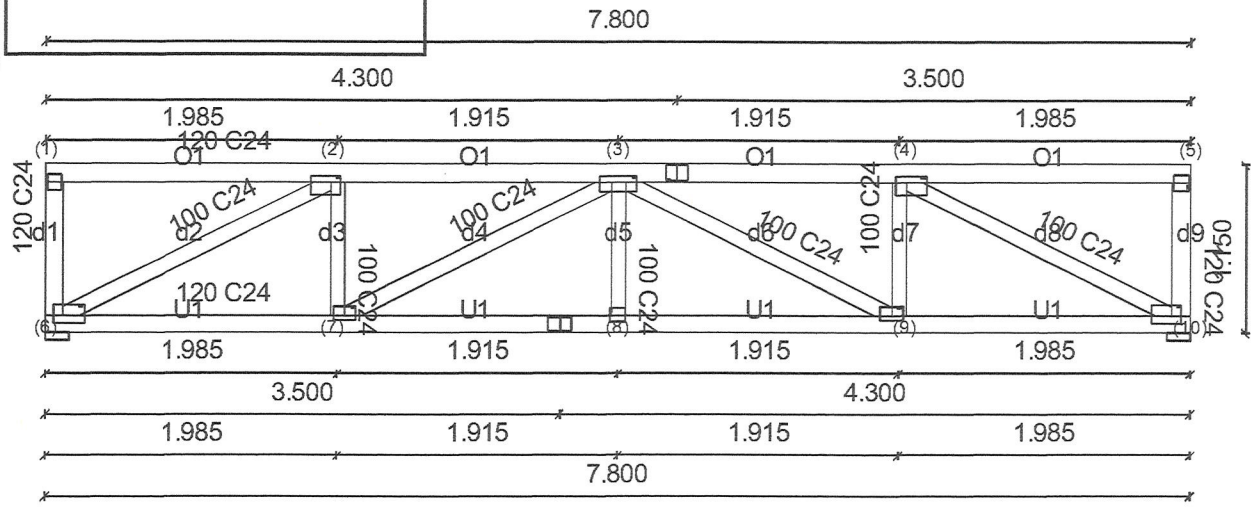


STIKI :

pas 1 WOLF15N 102/152 pri4300

pas 2 WOLF15N 102/152 pri3500

Teza nosilca :101.5kg



OB N.S. ! (ZG. PAS)

Zs

WOLFTRUSS8.91	ST. NOSILCEV: 1 ST. VECKRAT : 1 DEBEL.: 60 RAZMAK : 1000mm	OBTEZBE: posebne obt. IMPREGNACIJA Keines	,RAZMAK LET.: 500mm NAROCILO: os	STURM D.O.O., GOMILSKO 30 A, 3303 GOMILSKO OZN.NOSIL Zs 1kosov 25.9.2025 PROJEKT: OS Tabor MERILO 1:50 IZVEDBA : INVESTIT. : os Tabor	wolf SYSTEM

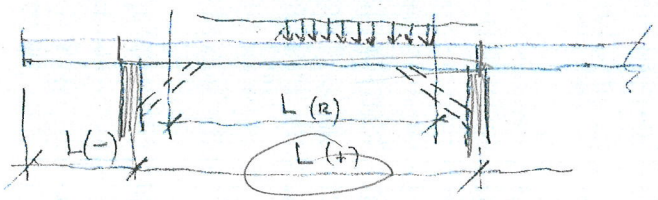
1. PRED IZDELAVO NOSILCEV MORA POOBLAŠČENA OSEBA IZDELOVALCA
2. KONTROLIRATI PODATKE UPOŠTEVANE V STAT. RAČUNU (MERE, NAKLON, ...) !
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4. IZRAČUN PO EC5 (DIN 1052)

DIMENZIONIRANJE LESENIH OSTREŠIJ

(les iglavcev II. kategorije - C24)

OŠT

Poz. V 01-vmesnik greda



razpon (L) (horizontalno)	2,50	m
vplivna širina (horizontalno)	1,00	m
naklon (α) (do horizontale)	0	°

podstrešje izkoriščeno? da

stalna obtežba	0,50 KN/m²		0,50 KN/m	1,35	0,68 KN/m
lastna teža			0,10 KN/m	1,35	0,14 KN/m
koristna obtežba (vertikalno)	1,00 KN/m²		1,00 KN/m	1,50	1,50 KN/m
skupna obtežba			1,60 KN/m	1,44	2,31 KN/m
		(0,60	1,00 KN/m)		
upogibni moment					1,80 KNm
prečna sila					2,89 KN

neto dimenzije lesa: **6,0** / **20,0** cm

W_d = 400 cm³ > 147

I_d = 4000 cm⁴ > 888

MSN

upogib:	$\sigma_{m,d}/f_{m,d}$ =	0,45 / 1,50	KN/cm²	0,30 <	1,0
strig:	$\tau_d/f_{v,d}$ =	0,05 / 0,15	KN/cm²	0,34 <	1,0

MSU

dejanski poves:	w_{inst} =	0,2 cm	<	0,8 (L/300)	0,22 <	1,0
končni poves:	w_{fin} =	0,3 cm	<	1,0 (L/250)	0,27 <	1,0

prečna povezanost grede? da

bočna zvrnitev ni merodajna

$\lambda_{rel,m}$ =

pohodni prostori? ne

vibracije ni merodajno

f_1 =

Upoštevano: $k_{mod}(1,2) = 0,8$

KONTROLA GLAVNIH P. TEMELJEV

(T1)

STREHA + STROP:	$2{,}2 \times 4{,}0$	$= 8{,}8$ kN/m¹
ŽID + OKNA	cca	$= 12{,}0$ "
PLOŠČA	$2 \times (8{,}0 \times 4{,}0)$	$\sim 64{,}0$ "
TEMELJ		$\sim 12{,}0$ "
		$\Sigma = 96{,}0$ kN/m¹ $\hat{=} 97{,}0$

USTREZA ŠIRINA 70 cm !

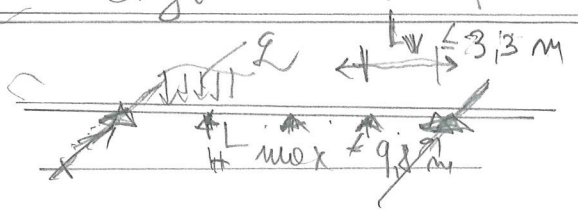
(T3)

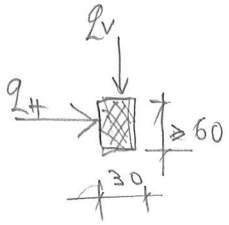
STREHA + STROP:	$2{,}2 \times 5{,}0$	$= 11{,}0$ kN/m¹
ŽID:	$40{,}5 + 0{,}3 \cdot 6{,}0 \cdot 3{,}5$	$\hat{=} 47{,}0$ "
PLOŠČA:	$2 \times (8{,}0 \times 5{,}0)$	$= 80{,}0$ "
TEMELJ:		$\hat{=} 13{,}0$ "
		151,0 kN/m¹ $\simeq$ 148 kN/m

USTREZA ŠIRINA 90 cm !

PRIMARNA KONTROLA OBT. Z VETROM + VERT. OBT.

ZUNANJI NOSILCI OSTREŠJA:





VERT. OBT.:

LASTNA: $0,3 \cdot 0,65 \cdot 25,0 = 4,9 \rightarrow 5,0$ kN/m¹

OD STREHE: - STALNA ≤ 5,0 "

- SNEG ≤ 5,0 "

$q_v \leq \underline{15,0}$ kN/m¹

$M_{max_V} \leq 15,0\ 3,3^2/10 = \pm 4,9 \rightarrow 5,0$ kNm

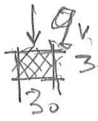
$F_{a\,max_V} \leq \frac{1,7 \cdot 5,0}{0,9 \cdot 0,5 \cdot 40} \leq 1,0\ cm^2 < F_a$ min

HOR. OBT.:

VETER: $0,5 \cdot 3,0 \leq 1,5$ kN/m¹ ; $M_{max_H} \leq 1,5 \cdot 9,8^2/8 = 18$ kNm

$F_{a\,max} \leq 1,7 \cdot 18,0/0,9 \cdot 0,25 \cdot 40 \leq \pm 3,4\ cm^2$

NOTRANJI NOSILCI OSTREŠJA



, $L_{max} \leq 3,5$ m

VERT. OBT.: ≤ 13,0 kN/m¹

$M_{max} \leq 13,0 \cdot 3,5^2/8 = 19,9$ kNm

$F_a \geq 1,7 \cdot 19,9/0,9 \cdot 0,25 \cdot 40 \geq 3,8\ cm^2$

Vhodni podatki - Konstrukcija okvirji

Shema nivojev

Naziv	z [m]	h [m]
+10,00 m	3.35	3.35

Naziv	z [m]	h [m]
+6,65 m	0.00	

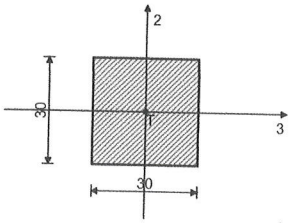
Tabele materialov

No	Naziv materiala	E[kN/m2]	μ	γ[kN/m3]	αt[1/C]	Em[kN/m2]	μm
1	C 25/30	3.100e+7	0.20	25.00	1.000e-5	3.100e+7	0.20

Seti gred

Set: 1 Prerez: b/d=30/30, Fiktivna ekscentričnost

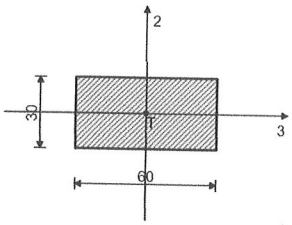
Mat.	A1	A2	A3	I1	I2	I3
1 - C 25/30	9.000e-2	7.500e-2	7.500e-2	1.141e-3	6.750e-4	6.750e-4



[cm]

Set: 2 Prerez: b/d=60/30, Fiktivna ekscentričnost

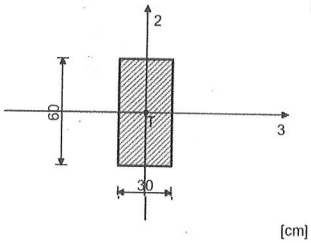
Mat.	A1	A2	A3	I1	I2	I3
1 - C 25/30	1.800e-1	1.500e-1	1.500e-1	3.708e-3	5.400e-3	1.350e-3



[cm]

Set: 3 Prerez: b/d=30/60, Fiktivna ekscentričnost

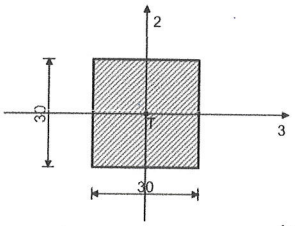
Mat.	A1	A2	A3	I1	I2	I3
1 - C 25/30	1.800e-1	1.500e-1	1.500e-1	3.708e-3	1.350e-3	5.400e-3



[cm]

Set: 4 Prerez: b/d=30/30, Fiktivna ekscentričnost

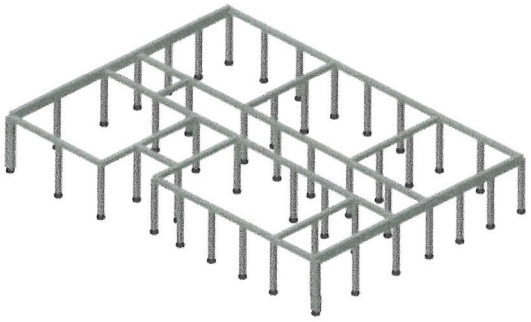
Mat.	A1	A2	A3	I1	I2	I3
1 - C 25/30	9.000e-2	7.500e-2	7.500e-2	1.141e-3	6.750e-4	6.750e-4



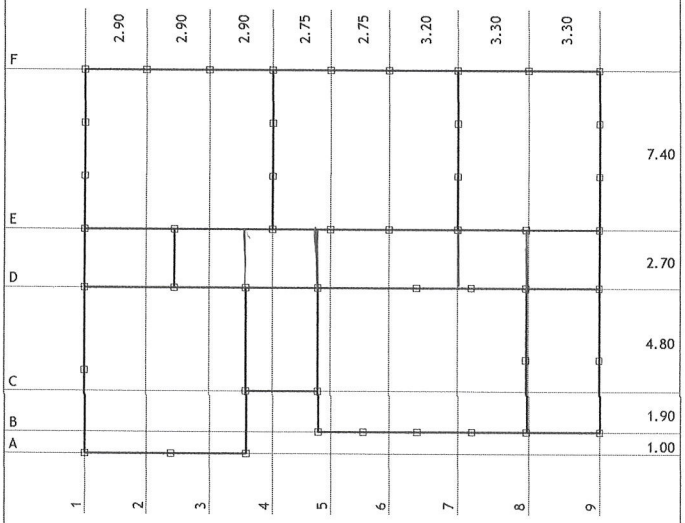
[cm]

Seti točkovnih podpor

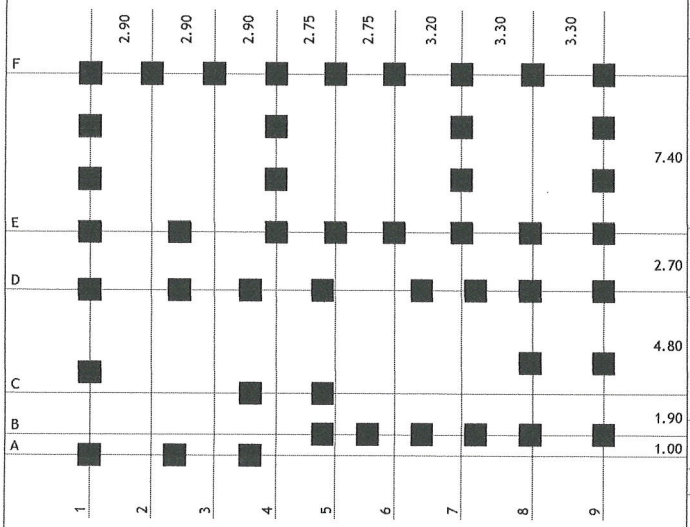
	K,R1	K,R2	K,R3	K,M1	K,M2	K,M3
1	1.000e+10	1.000e+10	1.000e+10			



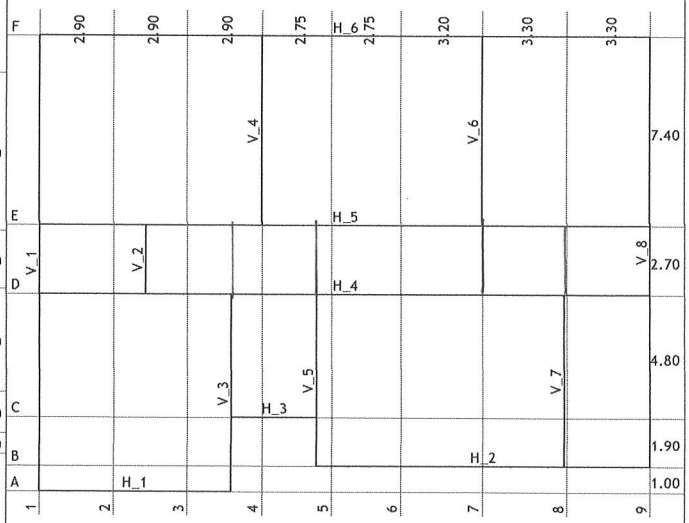
Izometrija



Nivo: +10,00 m [3.35 m]



Nivo: +6,65 m [0.00 m]



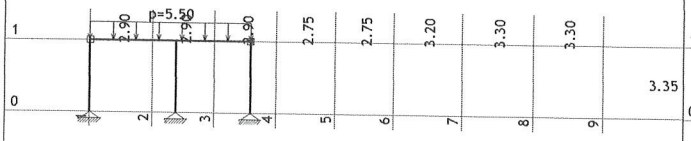
Dispozicija okvirjev

Vhodni podatki - Obtežba

Lista obtežnih primerov	
No	Naziv
1	LASTNA + STALNA (g)
2	SNEG
3	VETER X
4	VETER Y
5	POTRES X
6	POTRES Y
7	SRSS: I+V+0.3xVI
8	SRSS: I+0.3xV+VI

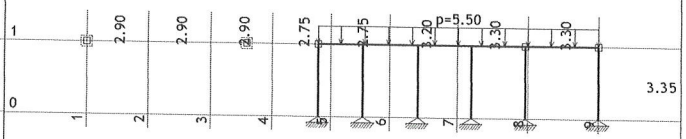
No	Naziv
9	Komb.: 1.35xI+1.5xII
10	Komb.: 1.5xIII
11	Komb.: 1.35xI+1.5xIII
12	Komb.: 1.5xIV
13	Komb.: 1.35xI+1.5xIV
14	Komb.: 1.35xI+1.5xII+0.9xIII
15	Komb.: 1.35xI+1.5xII+0.9xIV

Obt. 1: LASTNA + STALNA (g)



Okvir: H_1

Obt. 1: LASTNA + STALNA (g)



Okvir: H_2

Modalna analiza

Faktorji obtežb za preračun mas		
No	Naziv	Koeficient
1	LASTNA + STALNA (g)	1.00
2	SNEG	0.00
3	VETER X	0.00
4	VETER Y	0.00

Razporeditev mas po višini objekta					
Nivo	Z [m]	X [m]	Y [m]	Masa [T]	T/m2
+10,00 m	3.35	12.03	9.09	140.21	
+6,65 m	0.00	12.39	9.05	18.83	
Skupno:	2.95	12.07	9.08	159.04	

Položaj centra togosti po višini objekta			
Nivo	Z [m]	X [m]	Y [m]
+10,00 m	3.35	12.39	7.36
+6,65 m	0.00	12.39	7.36

Ekscentriciteta po višini objekta			
Nivo	Z [m]	eox [m]	eoy [m]
+10,00 m	3.35	0.36	1.72
+6,65 m	0.00	0.00	1.68

Nihajne dobe konstrukcije		
No	T [s]	f [Hz]
1	0.3688	2.7114
2	0.3464	2.8871
3	0.3401	2.9403

Seizmični preračun

Seizmični preračun: EC8 SLO

Kategorija tal:	B
Kategorija pomena:	III (γ=1.2)
Razmerje ag/g:	0.25
Faktor obnašanja:	1.5
Koeficient dušenja:	0.05
S:	1.2
Tb:	0.15
Tc:	0.5
Td:	2

Faktorji smeri potresa:

Naziv	Kx	Ky	Kz
POTRES X	1.000	0.000	0.000
POTRES Y	0.000	1.000	0.000

POTRES X

Nivo	Z [m]	Ton 1			Ton 2			Ton 3		
		Px [kN]	Py [kN]	Pz [kN]	Px [kN]	Py [kN]	Pz [kN]	Px [kN]	Py [kN]	Pz [kN]
+10,00 m	3.35	1.27	-27.54	-0.02	750.54	68.40	-0.86	8.23	-27.59	0.02
+6,65 m	0.00	0.03	-0.75	-0.00	20.01	1.96	-0.00	0.23	-0.86	0.00
	Σ=	1.30	-28.28	-0.02	770.55	70.36	-0.86	8.46	-28.45	0.02

Nivo	Z [m]	Vsi toni		
		Px [kN]	Py [kN]	Pz [kN]
+10,00 m	3.35	760.04	13.27	-0.86
+6,65 m	0.00	20.27	0.36	-0.00
	Σ=	780.31	13.63	-0.86

POTRES Y

Nivo	Z [m]	Ton 1			Ton 2			Ton 3		
		Px [kN]	Py [kN]	Pz [kN]	Px [kN]	Py [kN]	Pz [kN]	Px [kN]	Py [kN]	Pz [kN]
+10,00 m	3.35	-27.56	599.34	0.54	68.54	6.25	-0.08	-27.69	92.82	-0.08
+6,65 m	0.00	-0.72	16.22	0.00	1.83	0.18	-0.00	-0.76	2.88	-0.00
	Σ=	-28.28	615.57	0.54	70.36	6.43	-0.08	-28.45	95.70	-0.08

Nivo	Z [m]	Vsi toni		
		Px [kN]	Py [kN]	Pz [kN]
+10,00 m	3.35	13.28	698.41	0.38
+6,65 m	0.00	0.35	19.29	0.00
	Σ=	13.63	717.69	0.38

Faktorji participacije - relativno sodelovanje

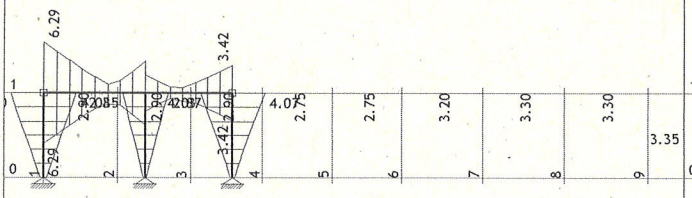
Ton \ Naziv	1. POTRES X	2. POTRES Y
1	0.002	0.858
2	0.987	0.009
3	0.011	0.133

Faktorji participacije - angažiranje mase

Ton	UX (%)	UY (%)	UZ (%)	ΣUX (%)	ΣUY (%)	ΣUZ (%)
1	0.14	65.78	0.00	0.14	65.78	0.00
2	82.34	0.69	0.00	82.48	66.47	0.00
3	0.90	10.23	0.00	83.38	76.69	0.00

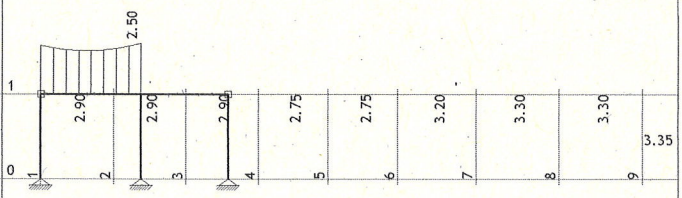
Dimenzioniranje (beton)

Merodajna obtežba: 7-9
EC 2 (EN 1992-1-1:2004), C 25, S500H



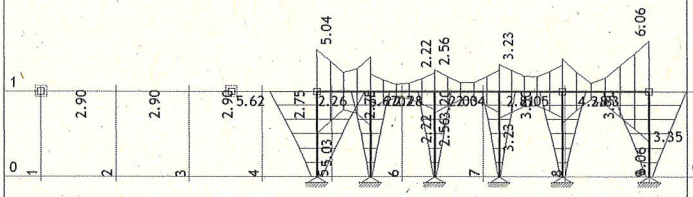
Okvir: H_1
Armatura v gredah: max Aa2/Aa1= 6.29 cm2

Merodajna obtežba: 7-9
EC 2 (EN 1992-1-1:2004), C 25, S500H



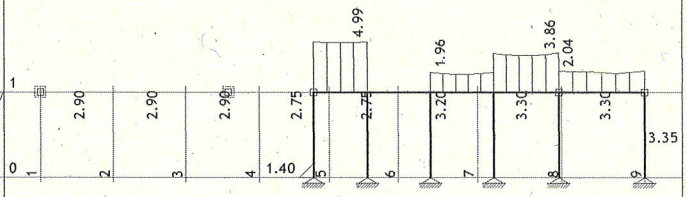
Okvir: H_1
Armatura v gredah: max Aa,st= 2.50 cm2

Merodajna obtežba: 7-9
EC 2 (EN 1992-1-1:2004), C 25, S500H



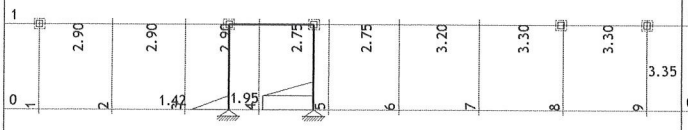
Okvir: H_2
Armatura v gredah: max Aa2/Aa1= 6.06 cm2

Merodajna obtežba: 7-9
EC 2 (EN 1992-1-1:2004), C 25, S500H



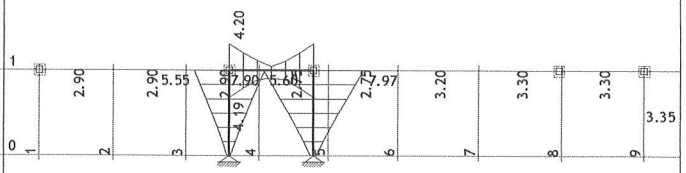
Okvir: H_2
Armatura v gredah: max Aa,st= 4.99 cm2

Merodajna obtežba: 7-9
EC 2 (EN 1992-1-1:2004), C 25, S500H



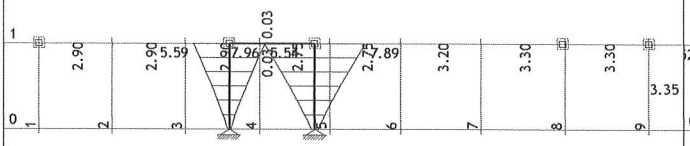
Okvir: H_3
Armatura v gredah: max Aa,st= 1.95 cm2

Merodajna obtežba: 7-9
EC 2 (EN 1992-1-1:2004), C 25, S500H



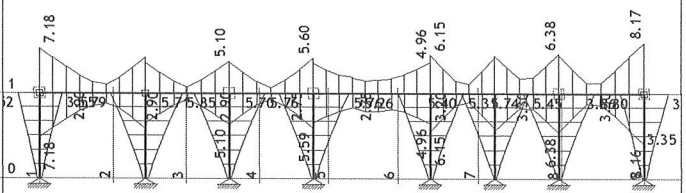
Okvir: H_3
Armatura v gredah: max Aa2/Aa1= 7.97 cm2

Merodajna obtežba: 7-9
EC 2 (EN 1992-1-1:2004), C 25, S500H



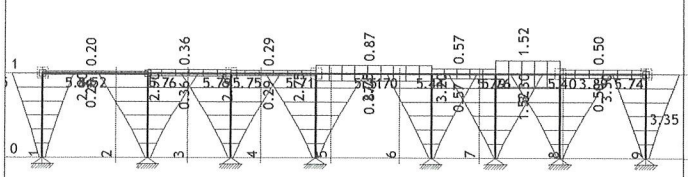
Okvir: H_3
Armatura v gredah: max Aa3/Aa4= 7.96 cm2

Merodajna obtežba: 7-9
EC 2 (EN 1992-1-1:2004), C 25, S500H



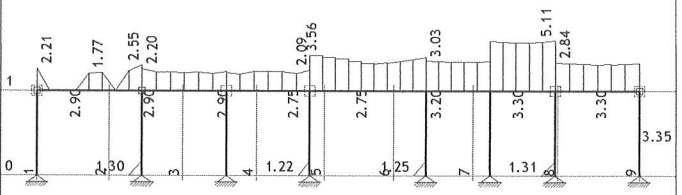
Okvir: H_4
Armatura v gredah: max Aa2/Aa1= 8.17 cm2

Merodajna obtežba: 7-9
EC 2 (EN 1992-1-1:2004), C 25, S500H



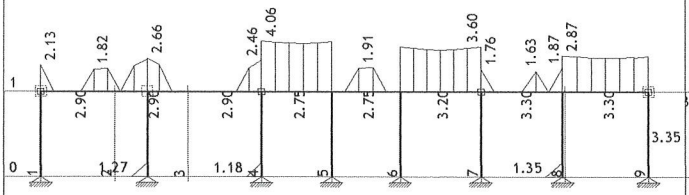
Okvir: H_4
Armatura v gredah: max Aa3/Aa4= 5.84 cm2

Merodajna obtežba: 7-9
EC 2 (EN 1992-1-1:2004), C 25, S500H



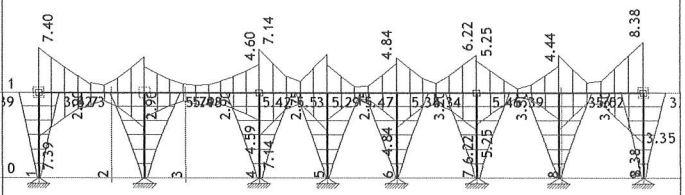
Okvir: H_4
Armatura v gredah: max Aa,st= 5.11 cm2

Merodajna obtežba: 7-9
EC 2 (EN 1992-1-1:2004), C 25, S500H



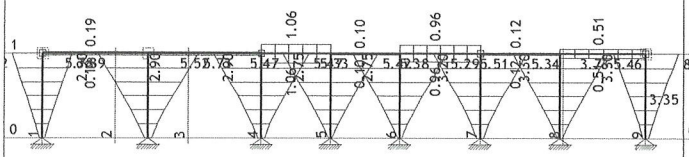
Okvir: H_5
Armatura v gredah: max Aa,st= 4.06 cm2

Merodajna obtežba: 7-9
EC 2 (EN 1992-1-1:2004), C 25, S500H



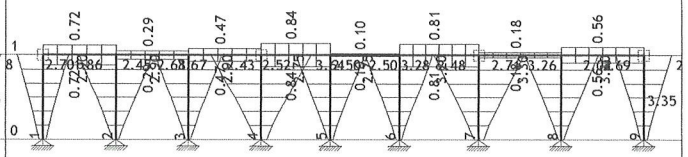
Okvir: H_5
Armatura v gredah: max Aa2/Aa1= 8.38 cm2

Merodajna obtežba: 7-9
EC 2 (EN 1992-1-1:2004), C 25, S500H



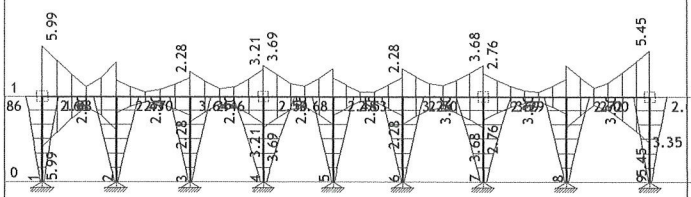
Okvir: H_5
Armatura v gredah: max Aa3/Aa4= 5.78 cm2

Merodajna obtežba: 7-9
EC 2 (EN 1992-1-1:2004), C 25, S500H



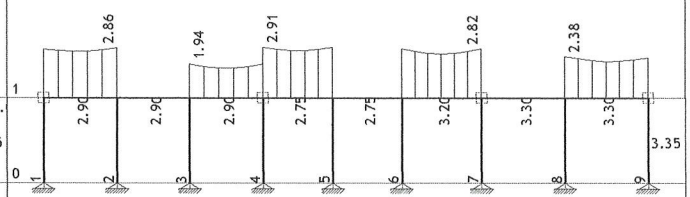
Okvir: H_6
Armatura v gredah: max Aa3/Aa4= 3.67 cm2

Merodajna obtežba: 7-9
EC 2 (EN 1992-1-1:2004), C 25, S500H



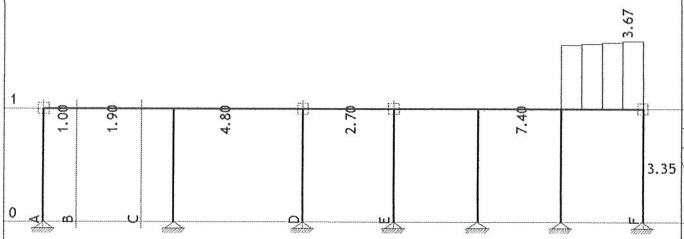
Okvir: H_6
Armatura v gredah: max Aa2/Aa1= 5.99 cm2

Merodajna obtežba: 7-9
EC 2 (EN 1992-1-1:2004), C 25, S500H



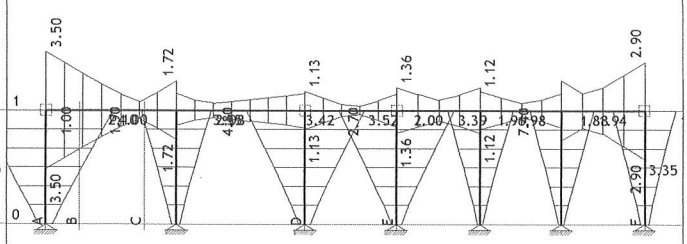
Okvir: H_6
Armatura v gredah: max Aa,st= 2.91 cm2

Merodajna obtežba: 7-9
EC 2 (EN 1992-1-1:2004), C 25, S500H



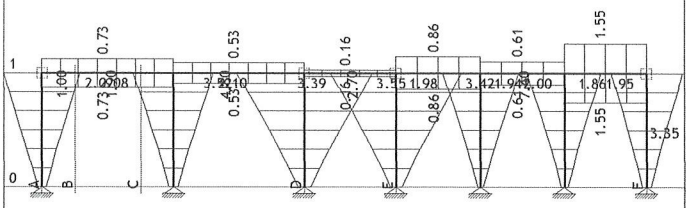
Okvir: V_1
Armatura v gredah: max Aa,st= 3.67 cm2

Merodajna obtežba: 7-9
EC 2 (EN 1992-1-1:2004), C 25, S500H



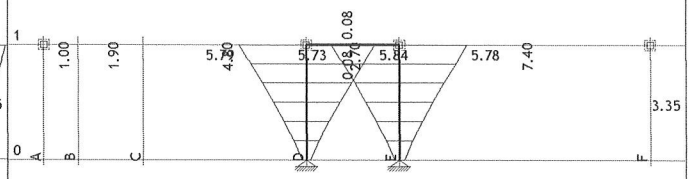
Okvir: V_1
Armatura v gredah: max Aa2/Aa1= 4.03 cm2

Merodajna obtežba: 7-9
EC 2 (EN 1992-1-1:2004), C 25, S500H



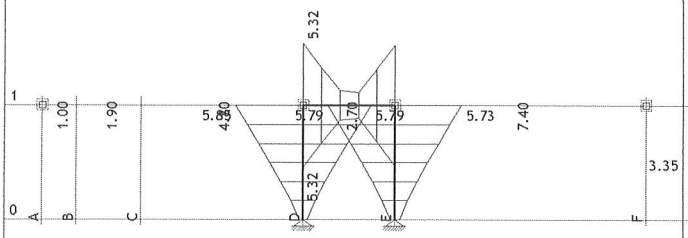
Okvir: V_1
Armatura v gredah: max Aa3/Aa4= 3.55 cm2

Merodajna obtežba: 7-9
EC 2 (EN 1992-1-1:2004), C 25, S500H



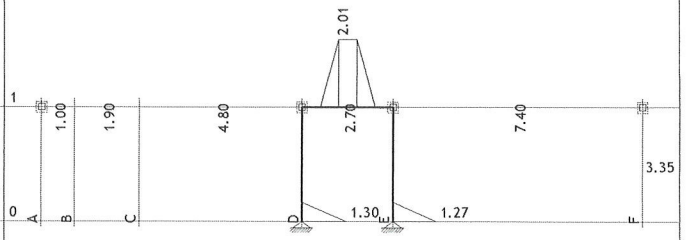
Okvir: V_2
Armatura v gredah: max Aa3/Aa4= 5.84 cm2

Merodajna obtežba: 7-9
EC 2 (EN 1992-1-1:2004), C 25, S500H



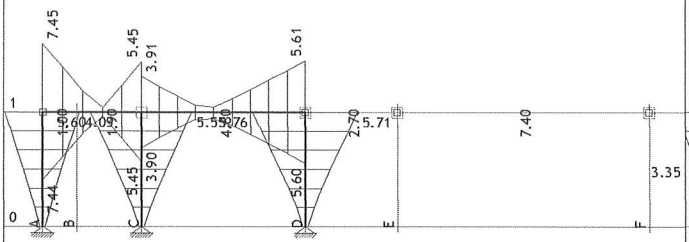
Okvir: V_2
Armatura v gredah: max Aa2/Aa1= 5.85 cm2

Merodajna obtežba: 7-9
EC 2 (EN 1992-1-1:2004), C 25, S500H



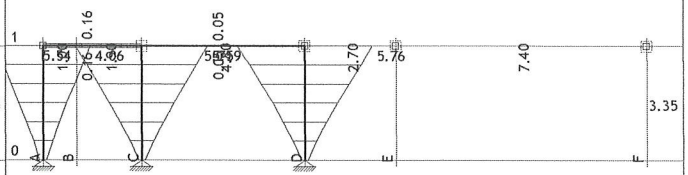
Okvir: V_2
Armatura v gredah: max Aa,st= 2.01 cm2

Merodajna obtežba: 7-9
EC 2 (EN 1992-1-1:2004), C 25, S500H



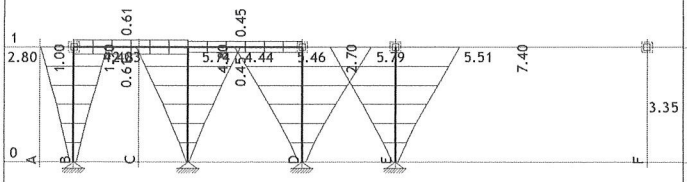
Okvir: V_3
Armatura v gredah: max Aa2/Aa1= 7.45 cm2

Merodajna obtežba: 7-9
EC 2 (EN 1992-1-1:2004), C 25, S500H



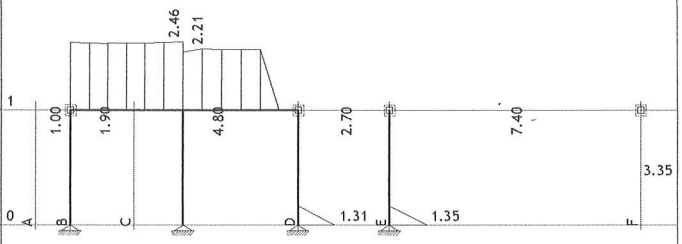
Okvir: V_3
Armatura v gredah: max Aa3/Aa4= 5.76 cm2

Merodajna obtežba: 7-9
EC 2 (EN 1992-1-1:2004), C 25, S500H



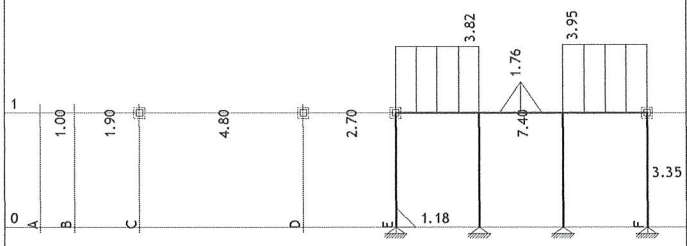
Okvir: V_7
Armatura v gredah: max Aa3/Aa4= 5.79 cm2

Merodajna obtežba: 7-9
EC 2 (EN 1992-1-1:2004), C 25, S500H



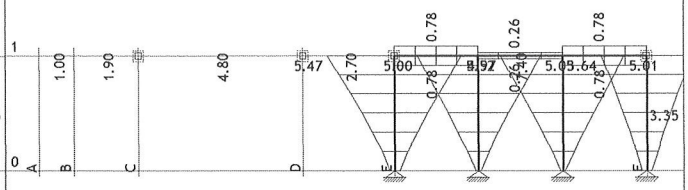
Okvir: V_7
Armatura v gredah: max Aa,st= 2.46 cm2

Merodajna obtežba: 7-9
EC 2 (EN 1992-1-1:2004), C 25, S500H



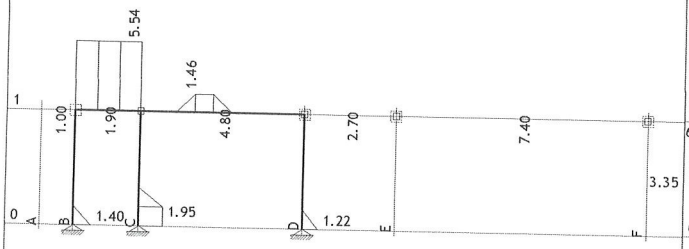
Okvir: V_4
Armatura v gredah: max Aa,st= 3.95 cm2

Merodajna obtežba: 7-9
EC 2 (EN 1992-1-1:2004), C 25, S500H



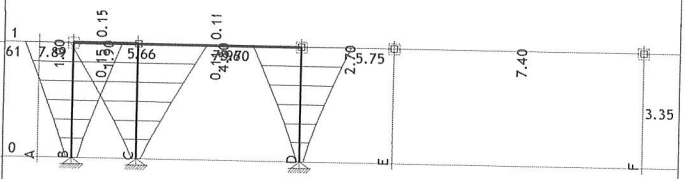
Okvir: V_4
Armatura v gredah: max Aa3/Aa4= 5.52 cm2

Merodajna obtežba: 7-9
EC 2 (EN 1992-1-1:2004), C 25, S500H



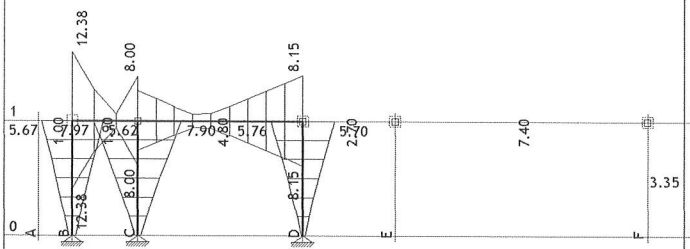
Okvir: V_5
Armatura v gredah: max Aa,st= 5.54 cm2

Merodajna obtežba: 7-9
EC 2 (EN 1992-1-1:2004), C 25, S500H



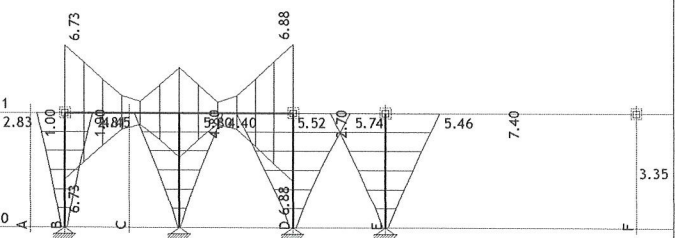
Okvir: V_5
Armatura v gredah: max Aa3/Aa4= 7.96 cm2

Merodajna obtežba: 7-9
EC 2 (EN 1992-1-1:2004), C 25, S500H



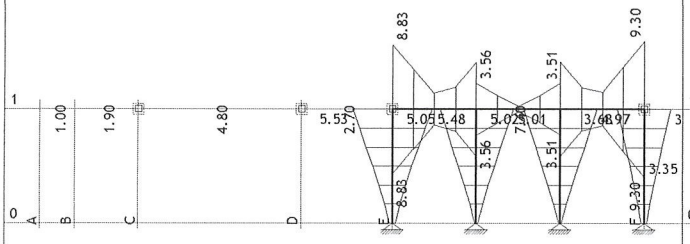
Okvir: V_5
Armatura v gredah: max Aa2/Aa1= 12.38 cm2

Merodajna obtežba: 7-9
EC 2 (EN 1992-1-1:2004), C 25, S500H



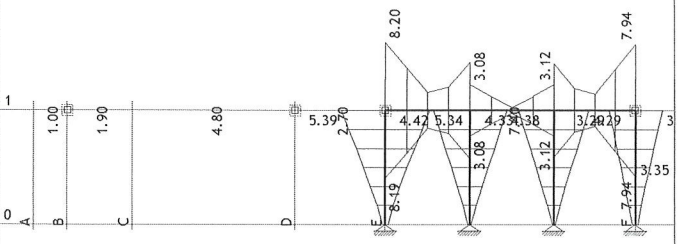
Okvir: V_7
Armatura v gredah: max Aa2/Aa1= 6.88 cm2

Merodajna obtežba: 7-9
EC 2 (EN 1992-1-1:2004), C 25, S500H



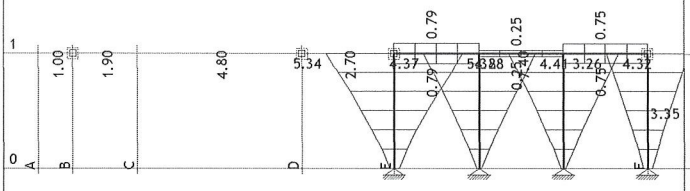
Okvir: V_4
Armatura v gredah: max Aa2/Aa1= 9.30 cm2

Merodajna obtežba: 7-9
EC 2 (EN 1992-1-1:2004), C 25, S500H



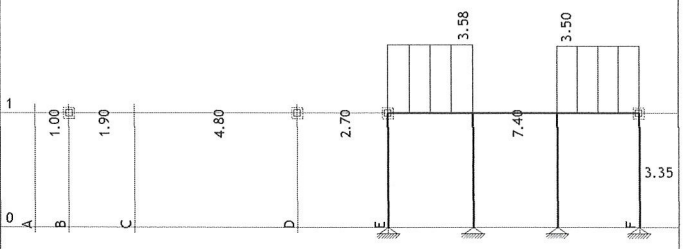
Okvir: V_6
Armatura v gredah: max Aa2/Aa1= 8.20 cm2

Merodajna obtežba: 7-9
EC 2 (EN 1992-1-1:2004), C 25, S500H



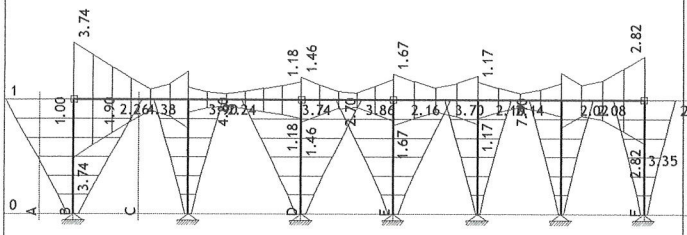
Okvir: V_6
Armatura v gredah: max Aa3/Aa4= 5.38 cm2

Merodajna obtežba: 7-9
EC 2 (EN 1992-1-1:2004), C 25, S500H



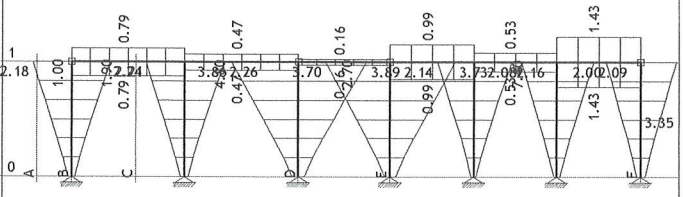
Okvir: V_6
Armatura v gredah: max Aa,st= 3.58 cm2

Merodajna obtežba: 7-9
EC 2 (EN 1992-1-1:2004), C 25, S500H



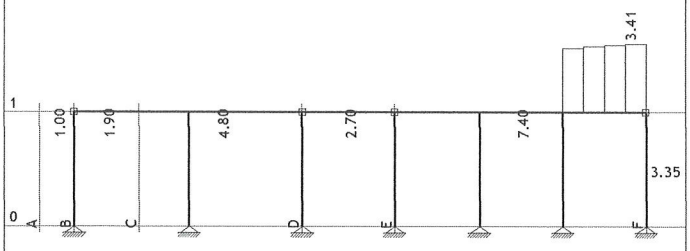
Okvir: V_8
Armatura v gredah: max Aa2/Aa1= 4.41 cm2

Merodajna obtežba: 7-9
EC 2 (EN 1992-1-1:2004), C 25, S500H



Okvir: V_8
Armatura v gredah: max Aa3/Aa4= 3.89 cm2

Merodajna obtežba: 7-9
EC 2 (EN 1992-1-1:2004), C 25, S500H



Okvir: V_8
Armatura v gredah: max Aa,st= 3.41 cm2

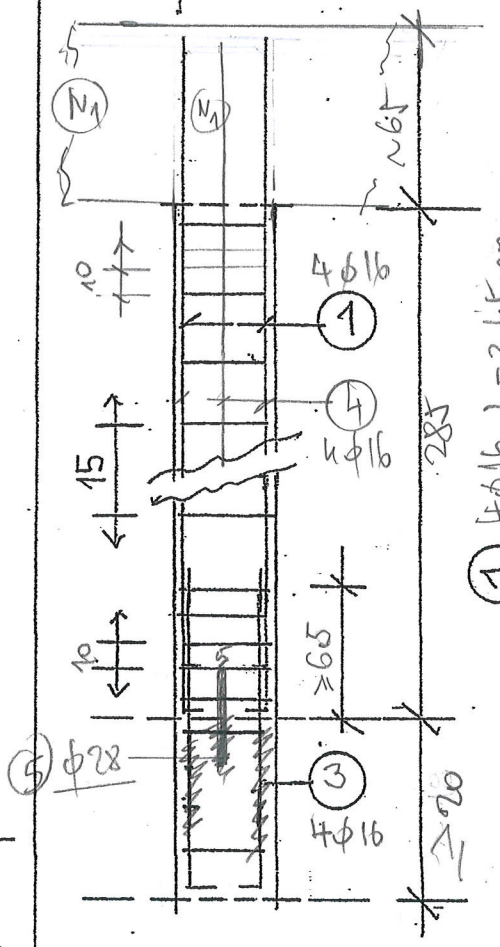
5x2
10K

C25/30 ⇐ MB 30 ; RA, MA 400 ⇒ S 400

ARMAT.

STEBER / POZ. (S1) — KOM 18

C. 25/30, S.500 B

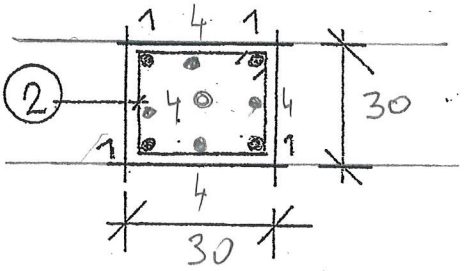


1 4ϕ16, L=3,45 m

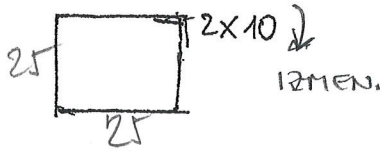
4 4ϕ16, L= 2,15 m

3 4ϕ16, L=0,85 m

5 1ϕ28, L=0,37



2 22ϕ8, L= 1,2 m1



ZA 1 KOM !

POZ.	KOM	ϕ	L	Kg/m¹	Kg ϕ≤12	Kg ϕ>12
1	4	16	3,45	1,64		22,6
2	22	8	1,2	0,41	11,0	
3	4	16	0,85	1,64		5,6
4	4	16	2,15	-"-		17,0
5	1	28	0,37	4,96		1,8
6						
kg					11,0	47,0 ZA 1 KOM
Σ					198,0	846,0 ZA 18 KOM
					ΣΣ	1044,0 kg

* OP! SIDRATI V KONSTR Z EPOXI LEPILOM!

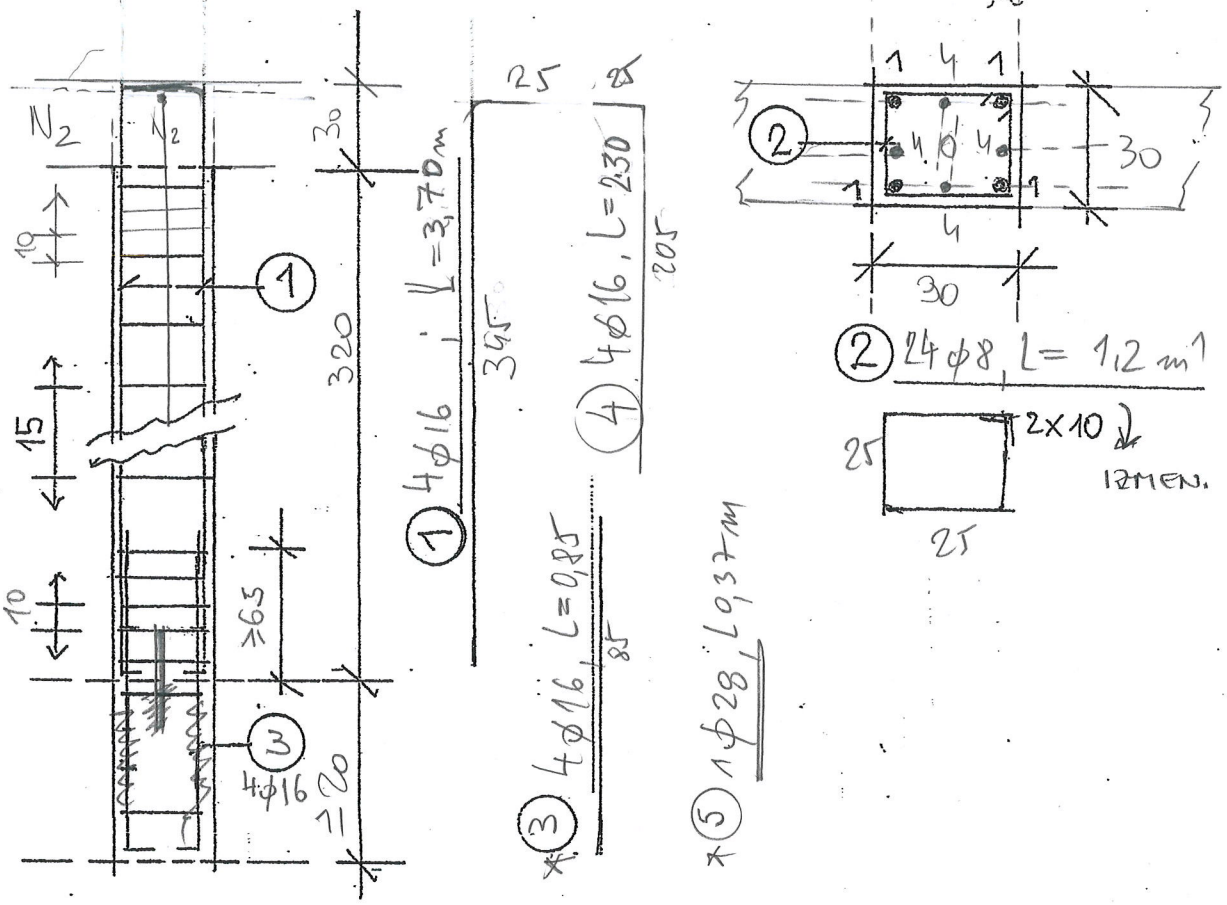
tkom
KONTROLA DIMENZIJ NA OBJEKTU!

*1.

5X2
10X

STEBER / POZ. S2 — KOM 25

C 25/30, S.500 P



ZA 1 KOM!

POZ.	KOM	φ	L	Kg/m1	Kg φ≤12	Kg φ>12
1	4	16	3,70	1,64		24,5
2	24	8	1,2	0,41	12,0	
3	4	16	0,85	1,64		5,5
4	4	16	2,30	-11-		15,0
5	1	28	0,37	4,96		2,0
6						
kg					12,0	47,0
Σ					300,0	1.175,0
					ΣΣ	1.475,0

ZA 1 KOM

ZA 25 KOM

Kg

* SIDRATI V KONSTR. Z EPOKSI LEPILOM!

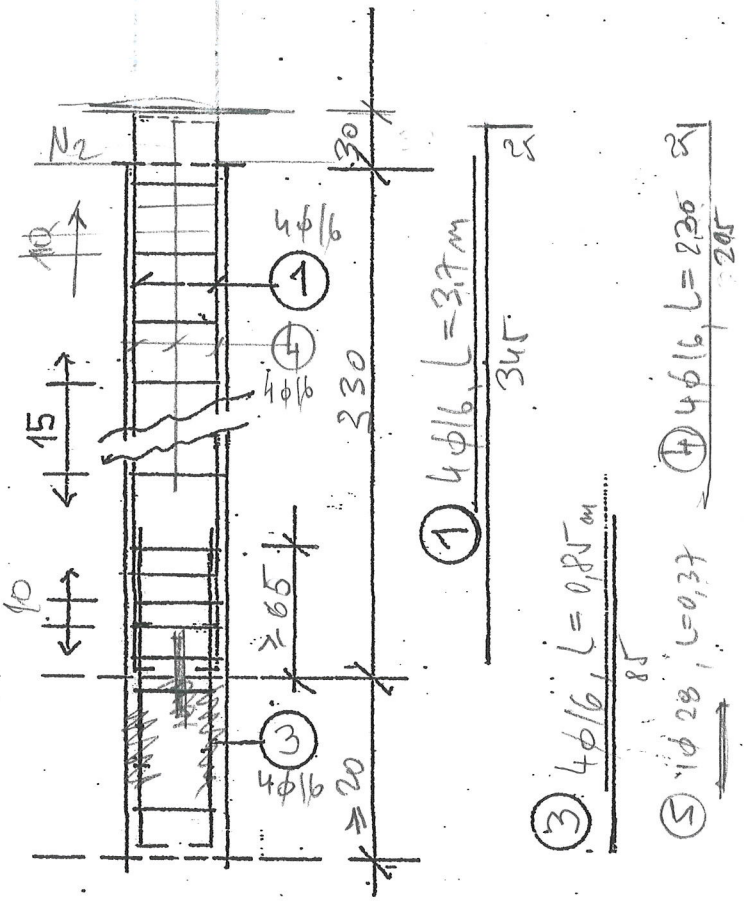
tkom
KONTROLA DIMENZIJ NA OBJEKTU!

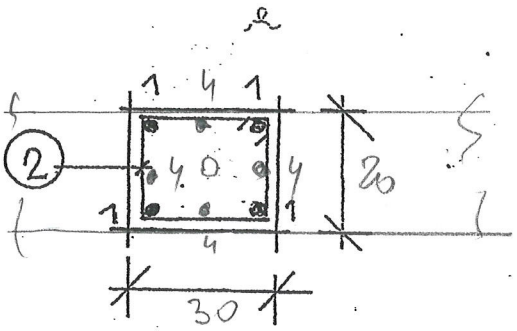
A2

5X2
10X

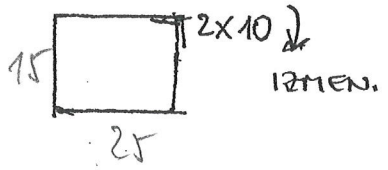
SŁUPY / POZ. S3 — KOM 4

C 25/30, S 500 P





2 24 φ8, L= 1,0 m



ZA 1 KOM !

POZ.	KOM	φ	L	Kg/m'	Kg φ≤12	Kg φ>12
1	4	16	3,7	1,64		24,5
2	24	8	1,0	0,41	10,0	
3	4	16	0,85	1,64		5,5
4	4	16	2,30	-"-		15,0
5	1	28	0,37	4,96		2,0
6.						
kg					10,0	47,0
Σ					40,0	188,0
ΣΣ						228,0

ZA 1 KOM

ZA 4 KOM

Kg

1kom

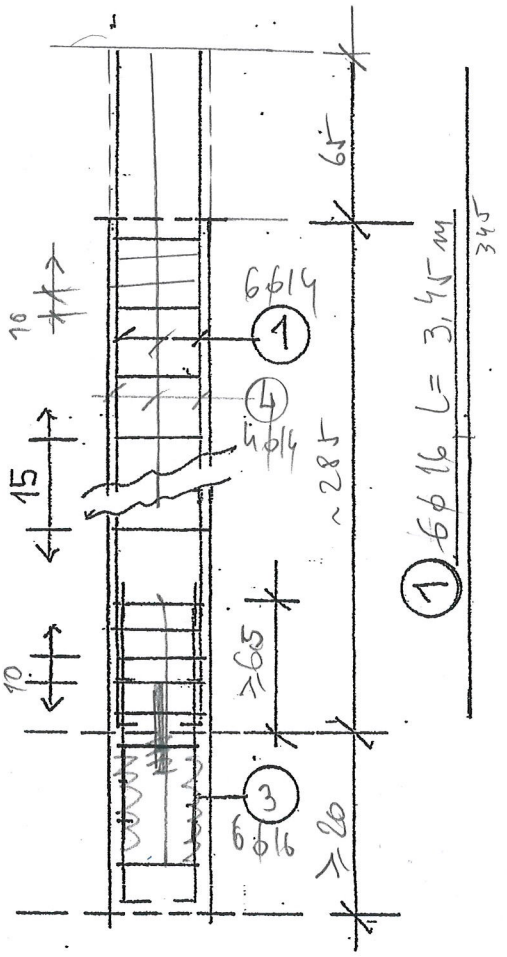
KONTROLA DIMENZJI NA OBIEKTU!

A-3

5X2
10X

STEBER / POZ. ⑤ — KOM 2

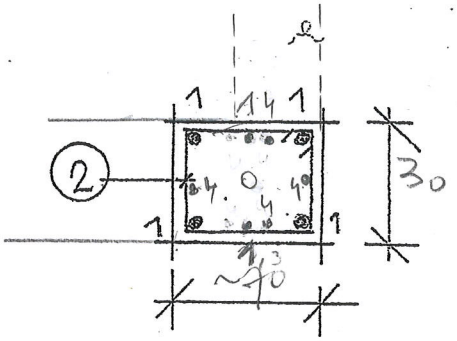
C 25/30, S 500 B



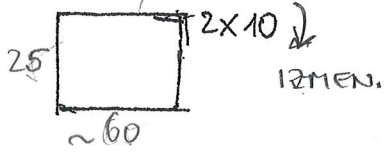
① 6φ16, L = 3,45 m

④ 4φ16, L = 2,15 m

③ 6φ16, L = 0,85 m



② 22φ8, L = 1,9 m



ZA 1 KOM !

POZ.	KOM	φ	L	Kg/m'	Kg φ≤12	Kg φ>12
1	6	16	3,45	1,64		34,0
2	22	8	1,9	0,41	17,5	
3	6	16	0,85	1,64		8,5
4	4	16	2,15	1,64		17,0
5	1	28	0,37	4,96		2,0
6.						
kg					17,5	60,5
Σ					35,0	121,0
ΣΣ						156,0

ZA 1 KOM

ZA 2 KOM

Kg

tkom
KONTROLA DIMENZIJ NA OBJEKTU!

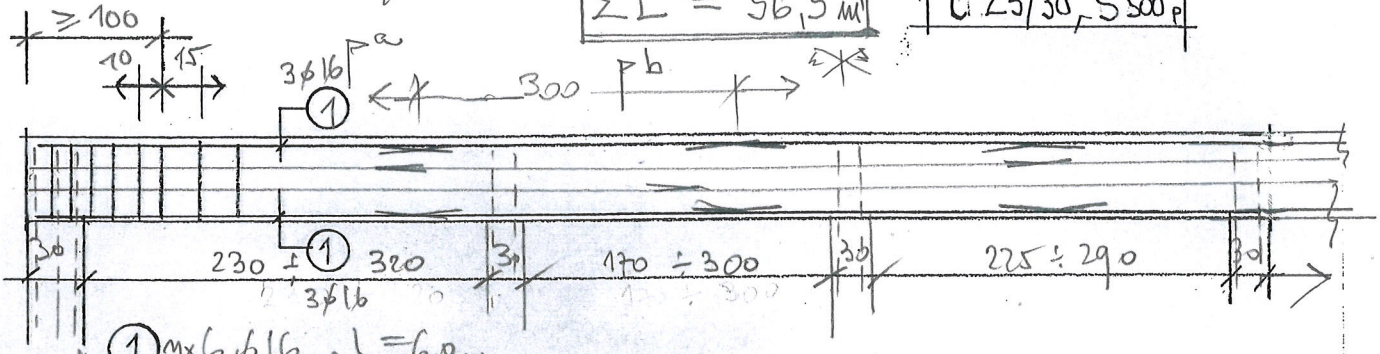
A4.

X

(10X)

NOSILEC /– POZ. (N) → KOM 1+1

$\Sigma L = 56{,}5\ m'$ | C 25/30, S 500 B

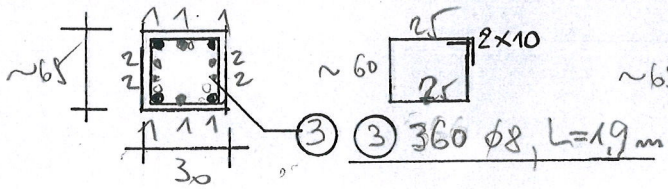


* (1) m × 6 φ16, L = 6,0 m / 600

* (2) m × 4 φ14, L = 6,0 m / 600

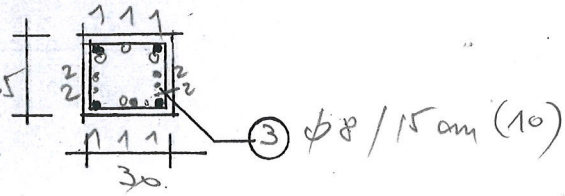
op.: PREKLOP DO ~50% NA 1 MESTU!

PREREZ a:



③ 360 φ8, L = 1,9 m

PREREZ b:



za 1 kom

POZ.	KOM	φ	L (m')	kg/m'	kg φ ≤ 12	kg φ > 12
* 1	66	16	6,0	1,64		650,0
* 2	44	14	6,0	1,25		330,0
3	360	8	1,9	0,41	280	
4						
5						
6						
7						
8						
Σ_1					280	980 ZA 1 KOM
Σ_2						ZA KOM
$\Sigma\Sigma$						1.260 kg

* RAZREZ NA GRI!

OPOMBA: KONTROLA NA OBJEKTU! DIMENZIJ

AT

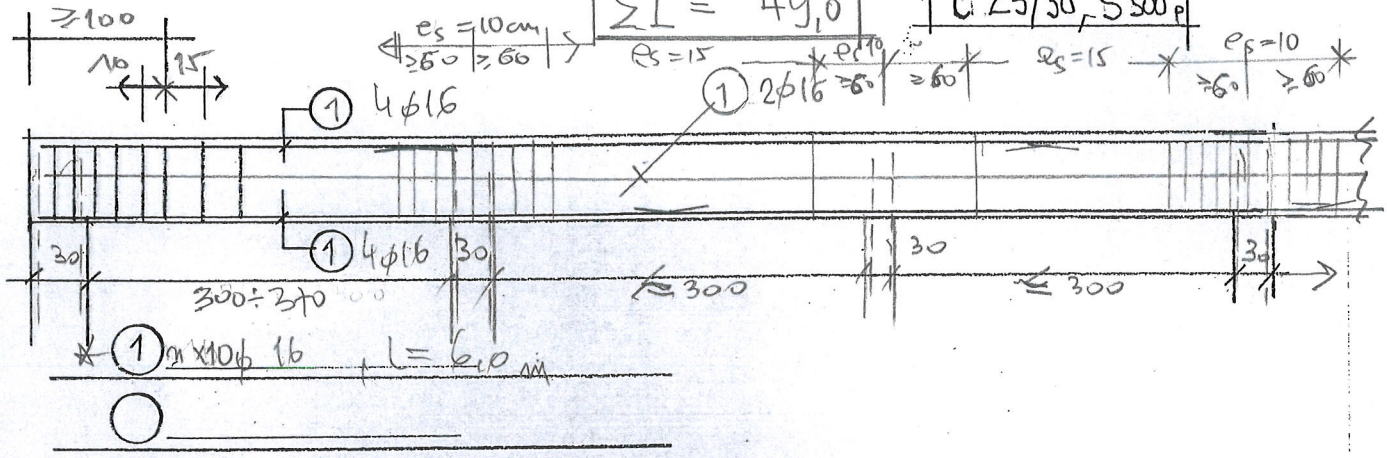
MB 25 = C 20/25
MB 30 = C 25/30 , RA 400 , NA 500 = S 500

X

(10X)

NOSILEC – POZ. (N2) → KOM 1+1

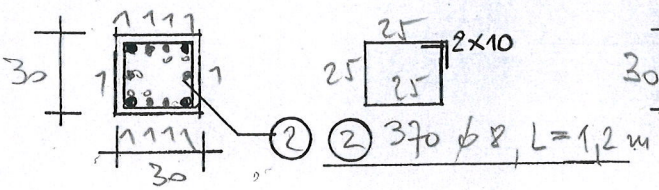
ΣL = 49,0 | C 25/30, S 500 B



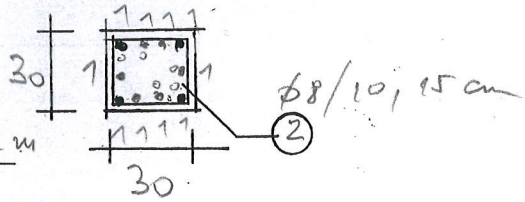
* PREKLOPI: max. 50% NA 1 MESTU!

* RAZREZ NA GRADBIŠČU

PREREZ a:



PREREZ b:



POZ.	KOM	φ	L (m')	kg/m'	kg φ≤12	kg φ>12
* 1	90	16	6,0	1,64		886,0
2	370	8	1,2	0,41	182	
3						
4						
5						
6						
7						
8						
Σ1					182	886,0 ZA 1 KOM
Σ2						ZA KOM
ΣΣ						1.068,0 kg

OPOMBA: KONTROLA NA OBJEKTU!

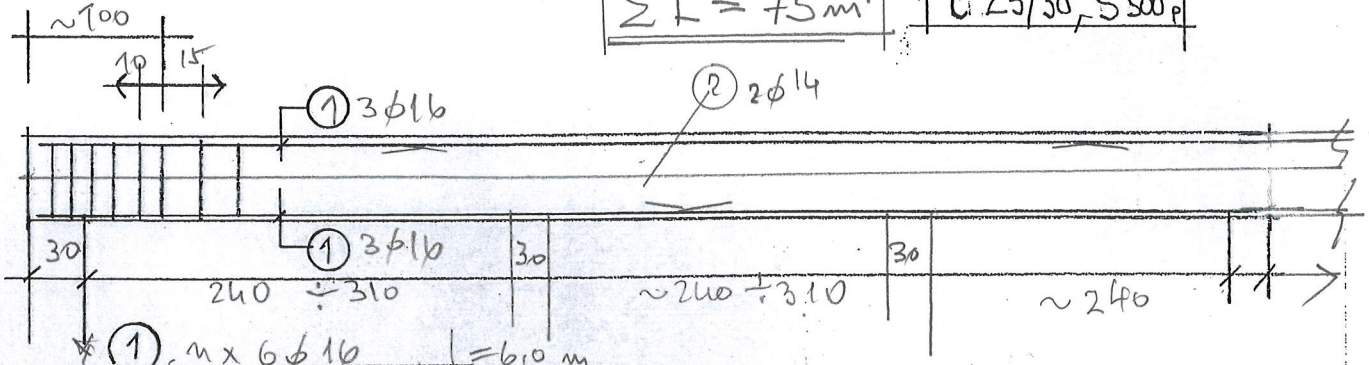
A6

X

(10X)

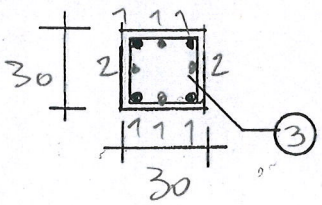
NOSILEC – POZ. (NP) – KOM 6

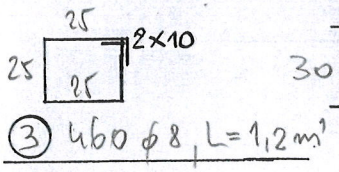
$\Sigma L \doteq 75\,m'$ | C 25/30, S 500 B



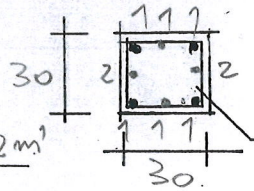
* PREKLOP ~50% na 1 mestu
* RAZREZ NA GRADBIŠČU!

PREREZ a:





PREREZ b:



③ ϕ8 (10) 15 cm

POZ.	KOM	ϕ	L (m')	kg/m'	kg ϕ≤12	kg ϕ>12
* 1	81	16	6,0	1,64		797
* 2	27	14	6,0	1,25		203
3	460	8	1,2	0,41	226	
4						
5						
6						
7						
8						
Σ_1					226	1.000 ZA 1 KOM
Σ_2						ZA KOM
$\Sigma\Sigma$						1.226 kg

OPOMBA: KONTROLA NA OBJEKTU!

A7

0x

IZVLEČEK ARMATURE S 500

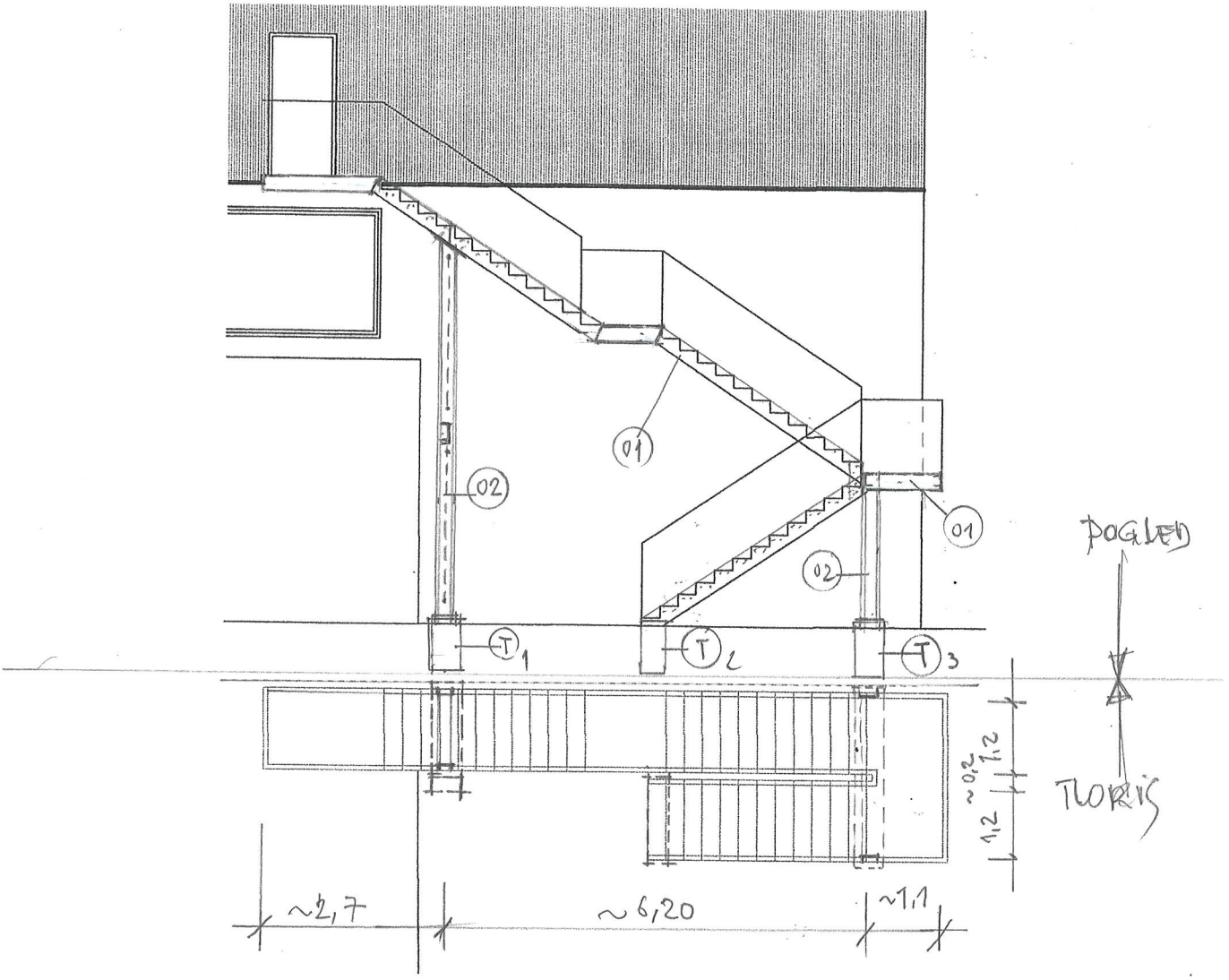
POZ.	OPIS		RA	MA	
S_1	STEBER		1.044		↑
S_2	"		1.475		
S_3	"		228		ŠOCA
S_4	"		156		
N_1	NOSILEC		1.260		
N_2	"		1.068		
N_p	"		1.226		↓
			6.457	kg	
T_1	TEMELJ		29		P: STOPNICE
T_2	"		25		
T_3	"		41		
		Σ	95	kg	
	SKUPAJ		6.552		kg

A8

POZICIJE

(a2) POŽARNE STOPNICE 1:100

1:100



ŠTURM d.o.o. 26.9.2025

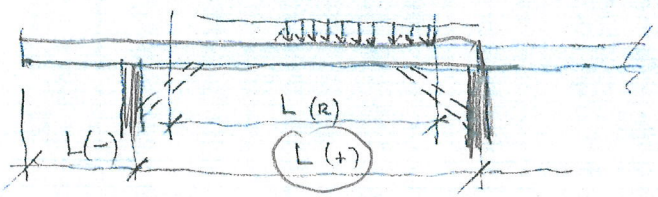
DIMENZIONIRANJE LESENIH OSTREŠIJ 01

(les iglavcev II. kategorije - C24)

OTŠ-STOPNICE

Poz. 01 greda

STAT.



razpon (L) (horizontalno)	6,10	m
vplivna širina (horizontalno)	0,65	m
naklon (α) (do horizontale)	0	°

JEKLO: $(f \leq l/300)$: $J_p \geq 15 \cdot 16{,}3 \cdot 6{,}1 \geq 1500\ cm^4$

USTREZA: UNP 200 ($1910\ cm^4$)

podstrešje izkoriščeno? da

stalna obtežba	2,00 KN/m²		1,30 KN/m	1,35	1,76 KN/m
lastna teža			0,25 KN/m	1,35	0,34 KN/m
koristna obtežba (vertikalno)	3,00 KN/m²		1,95 KN/m	1,50	2,93 KN/m
skupna obtežba			3,50 KN/m	1,43	5,02 KN/m
		(1,55	1,95 KN/m)		
upogibni moment			(16,3 KNm)	←	23,34 KNm
prečna sila			(10,6 KN)	←	15,30 KN

neto dimenzije lesa **20,0** / **28,0** cm

W_d = 2613 cm^3 > 2198

I_d = 36587 cm^4 > 28211

MSN

upogib:	$\sigma_{m,d}/f_{m,d}$ =	0,89 / 1,50 KN/cm²	0,60	<	1,0
strig:	$\tau_d/f_{v,d}$ =	0,06 / 0,15 KN/cm²	0,39	<	1,0

MSU

dejanski poves:	w_{inst} =	1,6 cm	<	2,0 (L/300)	0,77	< 1,0
končni poves:	w_{fin} =	2,3 cm	<	2,4 (L/250)	0,96	< 1,0

prečna povezanost grede? da

bočna zvrnitev ni merodajna

$\lambda_{rel,m}$ =

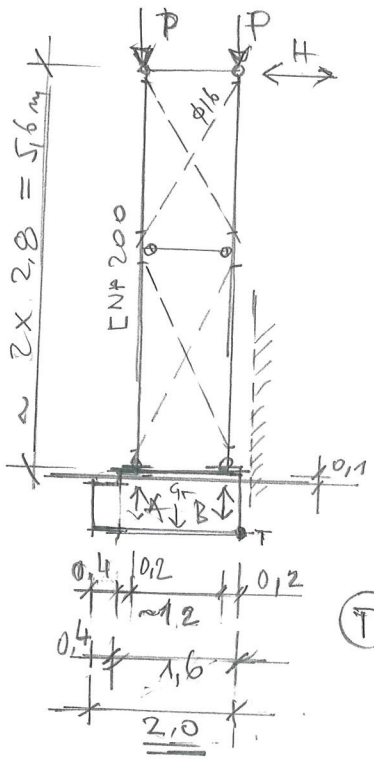
pohodni prostori? ne

vibracije ni merodajno

f_1 =

Upoštevano: $k_{mod}(1,2) = 0,8$

STEBRI STOPNIC: (02)



$P_{max} = 5,7 \times 3,5 = \underline{20,0\ kN}$

$H_{max} = 1,0 \times 5,7 = \pm 5,7\ kN$

$D_{max} \leq 15\ kN \rightarrow$ DIAGONALA Z BATEGO $\underline{\varnothing 16\ mm}$

$L_{UKL} = 5,6/2 = 3,8\ m$

$N_{max} \leq 80\ kN > \underline{26,6\ kN}$

$A_H = B_H = \pm 5,7 \cdot 5,6/1,2 = \pm 26,6\ kN > 20\ kN$

(T) TEMELJ: (1,6 X 0,4 X 1,0 m)

$N_{max} \leq 40\ kN$

$G_T = 16\ kN$

UPREVRNITEV: $V = 2P_{max}/2 = 20\ kN$

$_TM_H = 5,7 \cdot (5,6+1) = 37,6\ kNm$

$_TM_V = (40+16) \cdot 0,8 = 44,8\ kNm$

$V = \frac{44,8}{37,6} = 1,2 < 1,5$

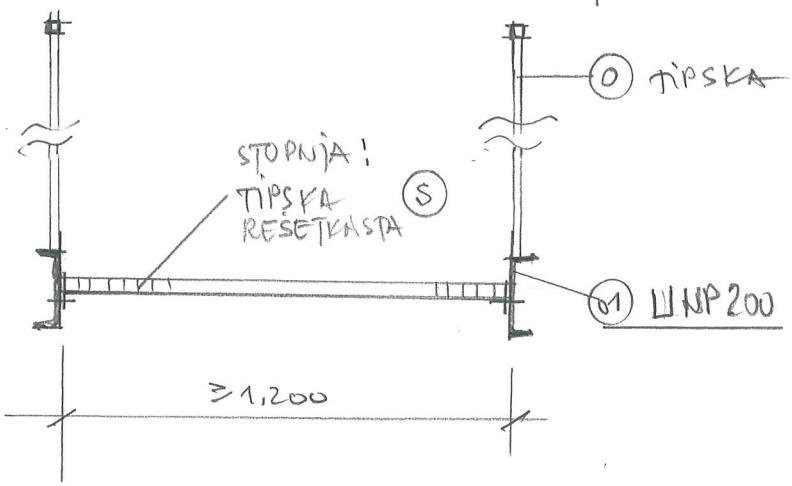
TEMELJ POVEČATI NA:

$\boxed{2,0 \times 0,4 \times 1,0}\ m^1$

SIDRANJE V TEMELJ: 4M16

JEKLO | STOPNIŠČNA RAMA

S235



POZ	OZNAKA	L (m)	kg/m'	kg	
01	U NP 200	~40,0	25,3	1.012	
S	STOPNJA-REŠ.	37, kom	–	37, kom	+ PODESTI
0	OGRAJA	~35,0	~10,0	350	
–	VEZNO SR.	OCENA	~10%	136	
			Σ	1.498	kg

STEBRI (S235)

POZ	OZN.	L (m)	kg/m'	kg	
02	E NP 200	~17,0	25,3	430	
–	POVEZAVE	OCENA	10%	~43	
			Σ	473	kg

Op.: VROČE CINKANO
KONTROLA NA OBJEKTU
DET. PRIPRAVI IZVAJALEC

Detalji



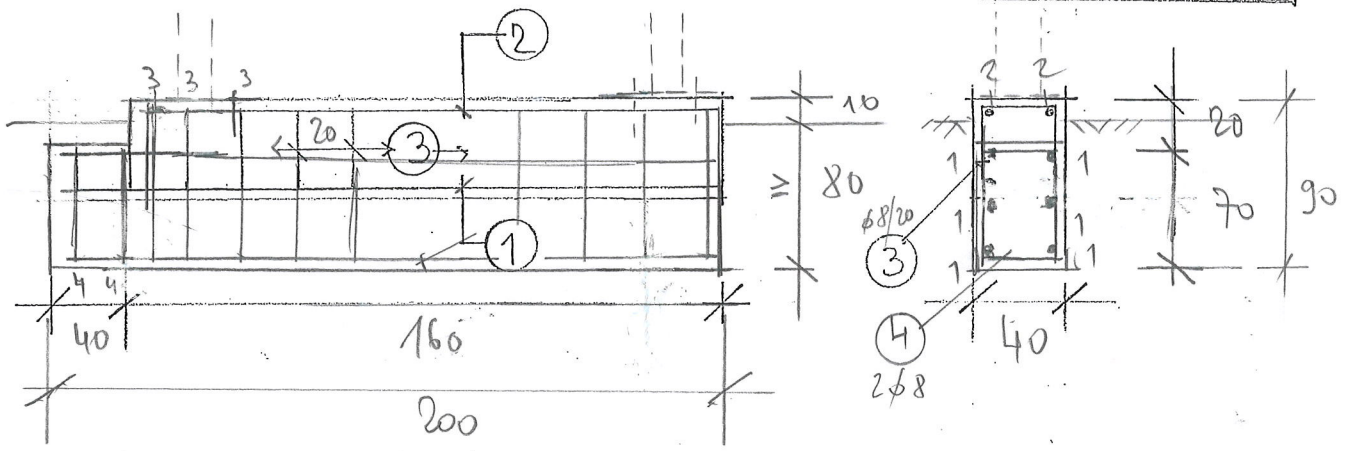




ARMATURA

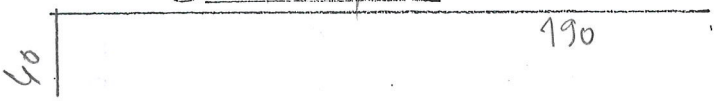
TEMELJ (40/200) / POZ. (T1) — KOM 1

C25/30; S500B

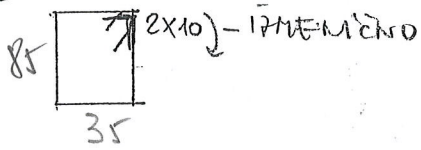


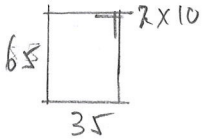
① 6 ϕ 14, L = 1,90 m

② 2 ϕ 14, L = 2,30 m



③ 9 ϕ 8 L = 2,6 m'





④ 2 ϕ 8, L = 2,20 m

ZA 1 KOM !

POZ	KOM	ϕ	L	kg/m'	kg ϕ ≤ 12	kg ϕ > 12
1	6	14	1,9	1,25		14,5
2	2	14	2,3	–"–		6,0
3	9	8	2,6	0,41	6,5	
4	2	8	2,2	–"–	2,0	
5						
6						
Σ:					8,5	20,5
Σ						
ΣΣ						29,0

ZA: 1 KOM

ZA ___ KOM

kg

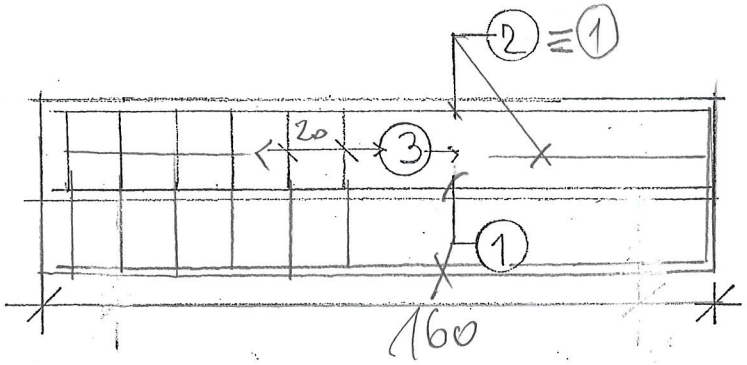
OPOMBA: KONTROLA NA OBJEKTU !

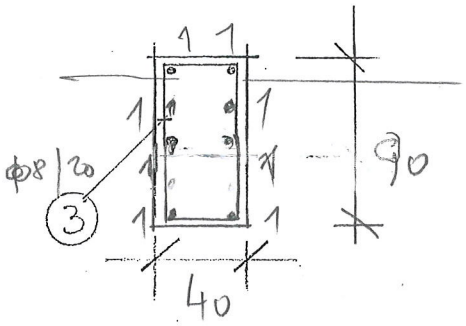
a2,1

5x
10x

TEMEL / POZ. (T)2 — KOM 1

C25/30; S500B

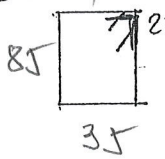




① 8 ϕ 14, L = 1,50 m

②

③ 9 ϕ 8, L = 2,6 m'



ZA 1 KOM!

POZ	KOM	ϕ	L	kg/m'	kg ϕ ≤ 12	kg ϕ > 12
1	8	14	1,5	1,25		15,0
2						
3	9	8	2,6	0,41	10,0	
4						
5						
6						
Σ:					10,0	15,0
Σ						
ΣΣ						25,0

ZA 1 KOM

ZA __ KOM

kg

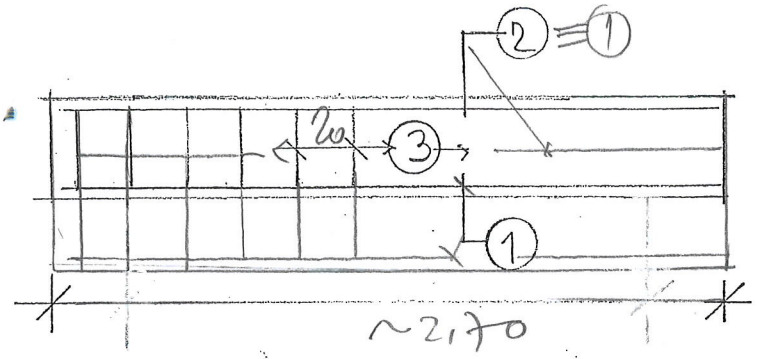
OPOMBA: KONTROLA NA OBJEKTU!

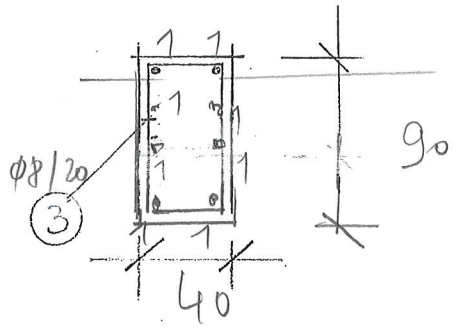
Q2,2

10X

TEMELJ / POZ. T3 — KOM 1

C25/30; S500B





① 8 Ø14, L = 2,60 m

②

③ 14 Ø8, L = 2,6 m'



ZA 1 KOM!

POZ	KOM	Ø	L	kg/m'	kg Ø≤12	kg Ø>12
1	8	14	2,6	1,25		26,0
2						
3	14	8	2,6	0,41	15,0	
4						
5						
6						
Σ:					15,0	26,0
Σ						
ΣΣ						41,0

ZA 1 KOM

ZA __ KOM

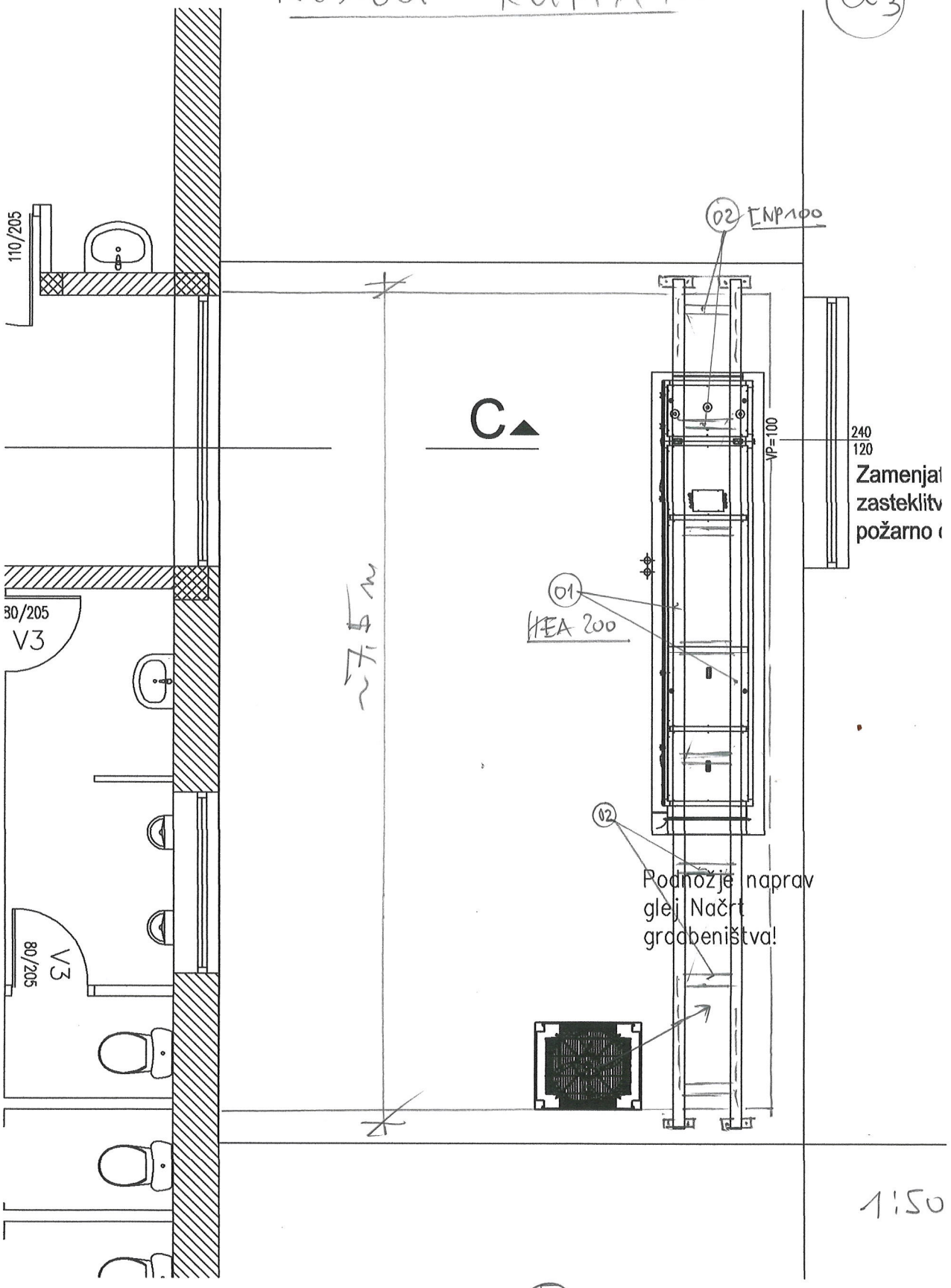
kg

T1 ÷ T3 ! Σkg = 95

OPOMBA: KONTROLA NA OBJEKTU! → SKUPNI IZVLEČEK GLEJ → ŠOLA! az,3

NOSILCI KLIMATA

(a3)



(a3) KLIMA NA JEKLENI KONSTR.

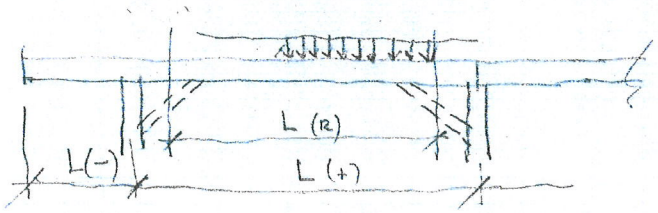
DIMENZIONIRANJE LESENIH OSTREŠIJ
(les iglavcev II. kategorije - C24)

11

OŠT-KLIMA

Poz. 01 greda

STATIKA



razpon (L) (horizontalno)	7,50	m
vplivna širina (horizontalno)	0,50	m
naklon (α) (do horizontale)	0	°

JEKLO ($f \leq L/300$):
$J \geq 15 \cdot 19{,}0 \cdot 7{,}5 \geq 2.140$ cm⁴ → USTREZA ~~HEA 200~~
(PREČKE: ENP 100)

podstrešje izkoriščeno? da

stalna obtežba	3,30 KN/m²		1,65 KN/m	1,35	2,23 KN/m
lastna teža			0,50 KN/m	1,35	0,68 KN/m
koristna obtežba (vertikalno)	1,00 KN/m²		0,50 KN/m	1,50	0,75 KN/m
skupna obtežba			2,65 KN/m	1,38	3,65 KN/m
		(2,15	0,50 KN/m)		
upogibni moment			(~19,0) ←		25,68 KNm
prečna sila					13,70 KN

neto dimenzije lesa: **20,0** / **30,0** cm

W_d =	3000 cm³	>	2760
I_d =	45000 cm⁴	>	39701

MSN

upogib:	$\sigma_{m,d}/f_{m,d}$ =	0,86 / 1,50 KN/cm²	0,57	<	1,0
strig:	$\tau_d/f_{v,d}$ =	0,05 / 0,15 KN/cm²	0,33	<	1,0

MSU

dejanski poves:	w_{inst} =	2,2 cm	<	2,5 (L/300)	0,88	<	1,0
končni poves:	w_{fin} =	3,7 cm	>	3,0 (L/250)	1,25	>	1,0

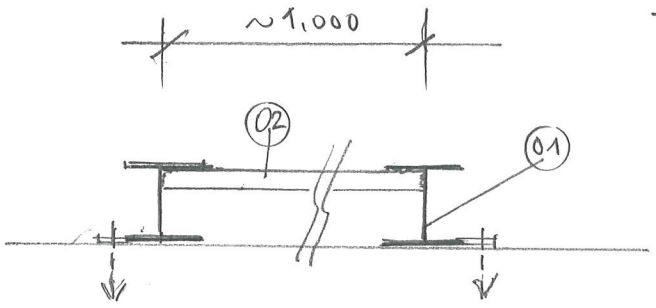
prečna povezanost grede? da
bočna zvrnitev ni merodajna
$\lambda_{rel,m}$ =

pohodni prostori? ne
vibracije ni merodajno
f_1 =

SIDRANJE: V A.B. ATIKO: 4 M 12

Upoštevano: $k_{mod}(1,2) = 0,8$

JEKLO : S 235



DOLŽINA : ~ 7.600

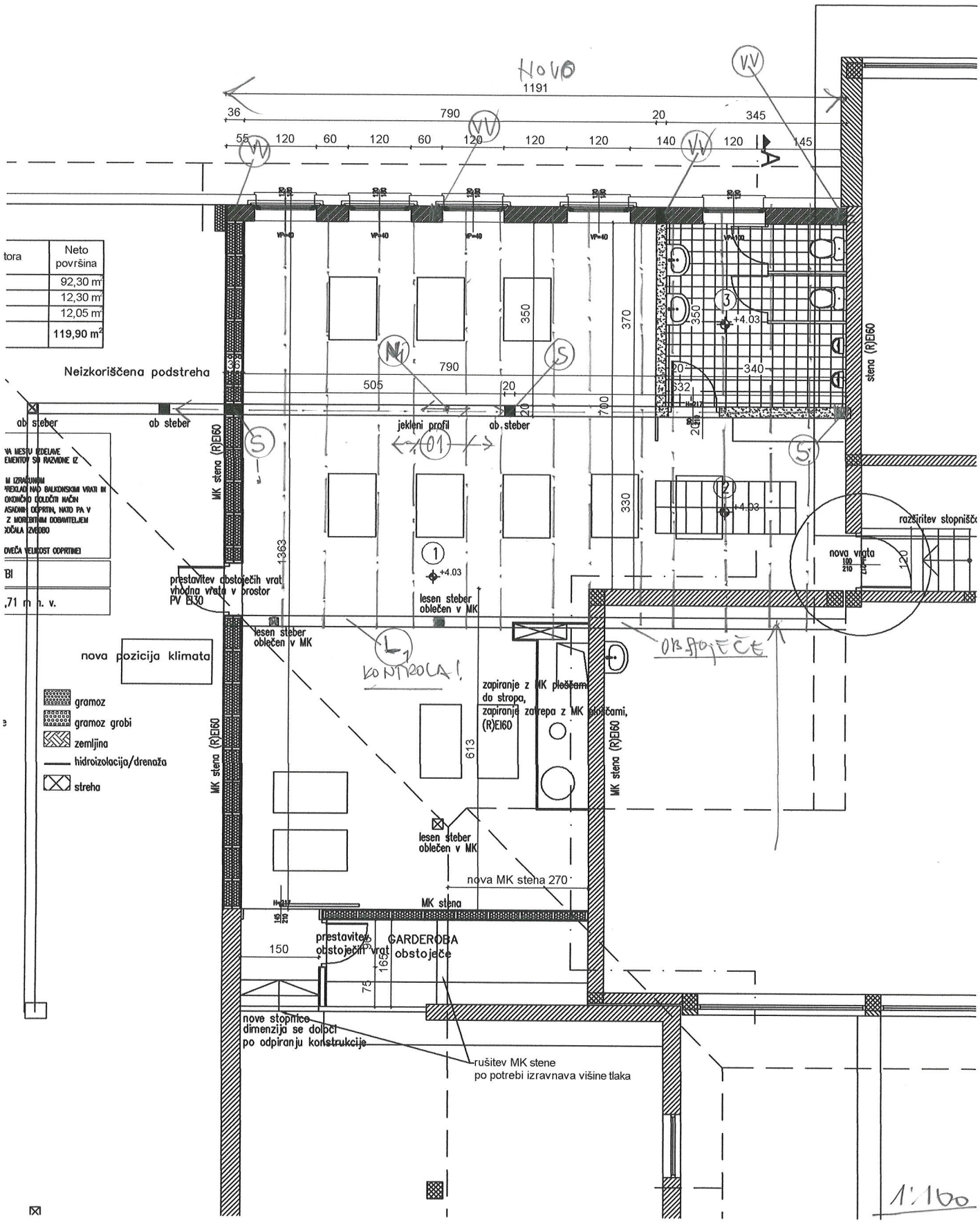
POZ	OPIS	(m) ΣL	kg/m	kg
01	HEA 200	15,2	42,3	643
02	UNP 100	8,0	10,6	85
—	VETNI EL.	~ 5%		30
			Σ	758 kg

OPT.: - VROČE CINKANO
- KONTROLA NA OBJEKTU
- DETAJLE PRIPRAVI IZVAJALEC

A3.1

pozicije

VRTEC (b)

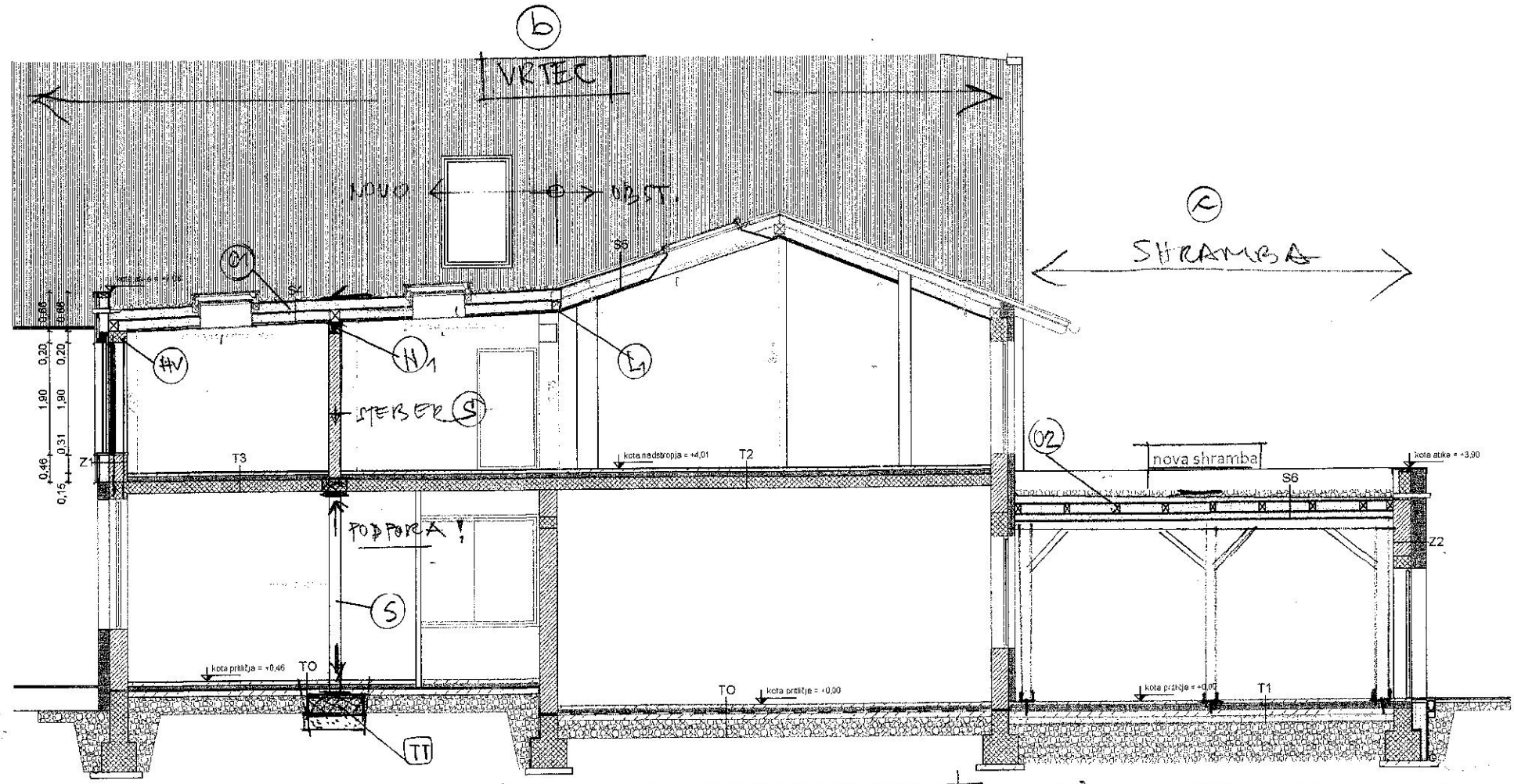


tora	Neto površina
	92,30 m²
	12,30 m²
	12,05 m²
	119,90 m²

(b) VRTEC

P1

1:100



OPOMBA: PRED IZVEDBO STEBROV PREVERITI PODPIRT V PRITLIČJU !!

KRITINA BREZ PRODCA !

1:100

b VRTEC IN c STRAMBA

POZ. ① / TEMELJI

$N_{max} \doteq N_L + N_q + N_T = 21 + 4 + 6 \doteq 31 kN$

NOSILNOST $\geq 100 kN/m^2$

$a_{1,T} = \sqrt{31/100} = 0,55 m \rightarrow 0,6/0,6 m$

TEMELJ: 60/60/30 cm + ~~ZBIT~~ VASUTJE $\geq$ 20 cm !

MIN. ARMATURA: SPODAJ MREŽA Q335 (ϕ8/10)

(ODREZEK) IPD

BETON (C 250)

OPOMBE**

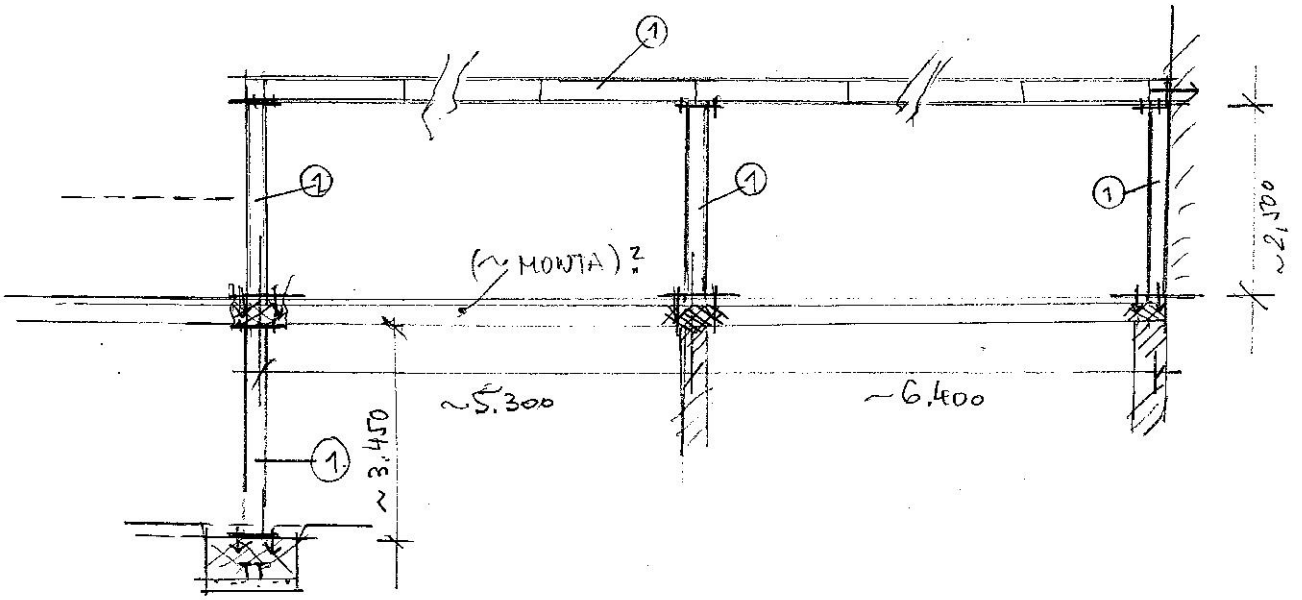
POTEK DEL:

- IZDELAVA T. TEMELJA (KOMPLET)!
- MONTAŽA STEBRA V PRITL. (S PODLIVANJEM)
- PODPIRANJE STROPNE KONSTR.
- BETONIRANJE LEŽIŠČA V OPEČNEM-REBR. STROPU, ~~ZA~~ STEBER V NADSTR.
- ~~MONTAŽA~~ STEBRA V NADSTROPJU!

JEKLO

POZ: (N)1, (S)

JEKLO; S 235



POZ	OZN.	Σ[km]	kg/m	kg
1	HEA 220	25.0	50,5	1.665
—	VEZNI EL.	~10%	OCENA	160 kg
			Σ	1.765 kg

OP.: - VROČE CINKANO
- KONTROLA MER NA OBJEKTU
- DETAJLE PRIPRAVI IZVAJALEC
- SODELUJ V KONSTRUKCIJAH (→)!
- GLEJ OPOMBE V STATIKI!

ŠTURM d.o.o. 25.9.2025

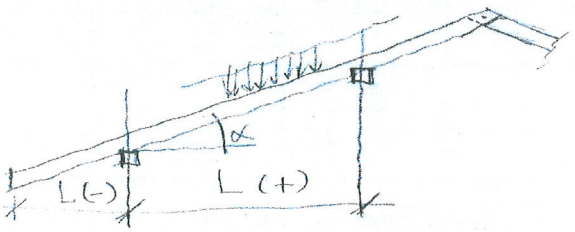
DIMENZIONIRANJE LESENIH OSTREŠIJ

(les iglavcev II. kategorije - C24)

OŠT-VRTEC

Poz. 01 špirovec

STATIKA



razpon (L) (horizontalno)					3,80	m		
razmik (e)					0,90	m		
naklon (α) (do horizontale)					5	°		
višina objekta (z)					6	m		
cona vetra (cv)	tlak vetra	0,43	KN/m²	1	0,00	KN/m²	1,5*0,6	0,00 KN/m²
kategorija terena				III				
nadmorska višina					320	m		
cona snega	sneg	1,54	KN/m²	A2	1,23	KN/m²	1,5	1,85 KN/m²
snegobrani?					da			
podstrešje izkoriščeno?					da			
razred uporabnosti					2			
stalna obtežba (poševno)		0,75	KN/m²		0,75	KN/m²	1,35	1,02 KN/m²
koristna obtežba (vertikalno)					0,10	KN/m²	1,50	0,15 KN/m²
skupna obtežba					2,09	KN/m²	1,45	3,02 KN/m²
					(1,88	KN/m)		
upogibni moment								4,90 KNm
prečna sila								5,16 KN

neto dimenzije lesa (b/h) — glej min. ▷ **12,0** / **16,0** cm

W_d =	512 cm³	>	475	
I_d =	4096 cm⁴	>	3665	

MSN

upogib:	$\sigma_{m,d}/f_{m,d}$ =	0,96 / 1,50	KN/cm²	0,64	< 1,0
strig:	$\tau_d/f_{v,d}$ =	0,06 / 0,15	KN/cm²	0,38	< 1,0

MSU

dejanski poves:	w_{inst} =	1,1 cm	<	1,3 (L/300)	0,89	< 1,0
končni poves:	w_{fin} =	1,6 cm	>	1,5 (L/250)	1,07	> 1,0

prečna povezanost špirovca?		da
bočna zvrnitev	ni merodajna	
$\lambda_{rel,m}$ =		
pohodni prostori?		ne
vibracije	ni merodajno	
f_1 =		

Upoštevano: $k_{mod}(1,2) = 0,8$; $k_{mod}(3) = 0,65$

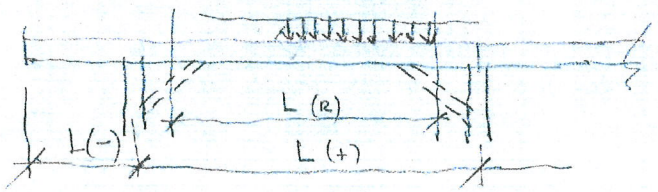
ŠTURM d.o.o. 25.9.2025

DIMENZIONIRANJE LESENIH OSTREŠIJ

(les iglavcev II. kategorije - C24)

OŠT-V

Poz. N1 lega



razpon (L) (horizontalno)	6,30	m
vplivna širina (horizontalno)	3,80	m
naklon (α) (do horizontale)	0	°

Jeklo * ($f \leq l/300$)

$J_p \geq 15 \cdot M \cdot 6,3 \Rightarrow 15 \cdot 41,7 \cdot 6,3 \geq 3.940\ cm^4$

POZ. (5) → ENAKO: USTREZA; HEA 220

lastna teža lege	0,30 KN/m		0,30 KN/m	1,35	0,41 KN/m
koristna obtežba (vertikalno)			0,10 KN/m	1,50	0,15 KN/m
skupna obtežba			8,33 KN/m	1,44	12,02 KN/m
		(3,16	5,17 KN/m)		
upogibni moment			(41,7)		59,65 KNm
prečna sila					37,87 KN

neto dimenzije lesa (b/h) **20,0** / **38,0** cm

W_d = 4813 cm^3 > 4545

I_d = 91453 cm^4 > 83921

MSN

upogib:	$\sigma_{m,d}/f_{m,d}$ =	1,24 / 1,50	KN/cm²	0,83	<	1,0
strig:	$\tau_d/f_{v,d}$ =	0,11 / 0,15	KN/cm²	0,71	<	1,0

MSU

dejanski poves:	w_{inst} =	1,7 cm	<	2,1 (L/300)	0,81	<	1,0
končni poves:	w_{fin} =	2,5 cm	<	2,5 (L/250)	0,98	<	1,0

prečna povezanost lege? da

bočna zvrnitev ni merodajna

$\lambda_{rel,m}$ =

pohodni prostori? ne

vibracije ni merodajno

f_1 =

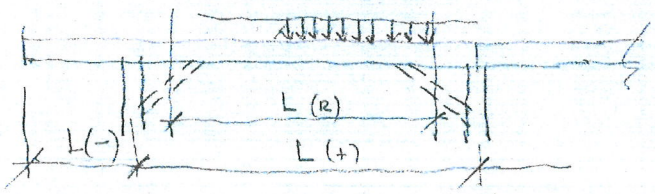
Upoštevano: $k_{mod}(1,2) = 0,8$; $k_{mod}(3) = 0,65$

DIMENZIONIRANJE LESENIH OSTREŠIJ

(les iglavcev II. kategorije - C24)

OŠT-V

Poz. L1 lega -OBSTOJEČA → KONTROLA DIMENZIJ!



razpon (L) (horizontalno)	3,00	m
vplivna širina (horizontalno)	3,50	m
naklon (α) (do horizontale)	0	°

lastna teža lege	0,30 KN/m		0,30	KN/m	1,35	0,41 KN/m
koristna obtežba (vertikalno)			0,10	KN/m	1,50	0,15 KN/m
skupna obtežba			7,70	KN/m	1,44	11,12 KN/m
		(2,94	4,77	KN/m)		
upogibni moment						12,51 KNm
prečna sila						16,68 KN

neto dimenzije lesa (b/h) MIN 18,0 / 18,0 cm

W_d =	972	cm^3	>	942
I_d =	8748	cm^4	>	8346

MSN

upogib:	$\sigma_{m,d}/f_{m,d}$ =	1,29 / 1,50	KN/cm²	0,86	<	1,0
strig:	$\tau_d/f_{v,d}$ =	0,11 / 0,15	KN/cm²	0,74	<	1,0

MSU

dejanski poves:	w_{inst} =	0,8 cm	<	1,0 (L/300)	0,84	< 1,0
končni poves:	w_{fin} =	1,2 cm	>	1,2 (L/250)	1,02	> 1,0

prečna povezanost lege?		da
bočna zvrnitev	ni merodajna	
$\lambda_{rel,m}$ =		
pohodni prostori?		ne
vibracije	ni merodajno	
f_1 =		

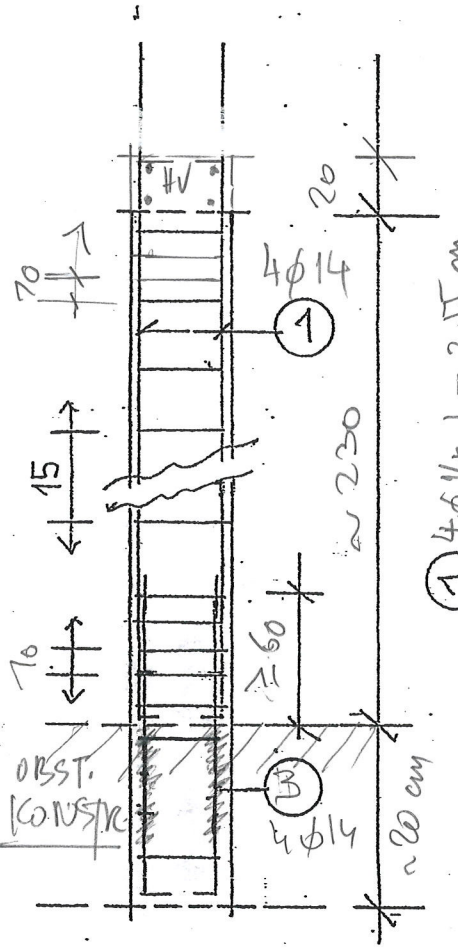
Upoštevano: $k_{mod}(1,2) = 0,8$; $k_{mod}(3) = 0,65$

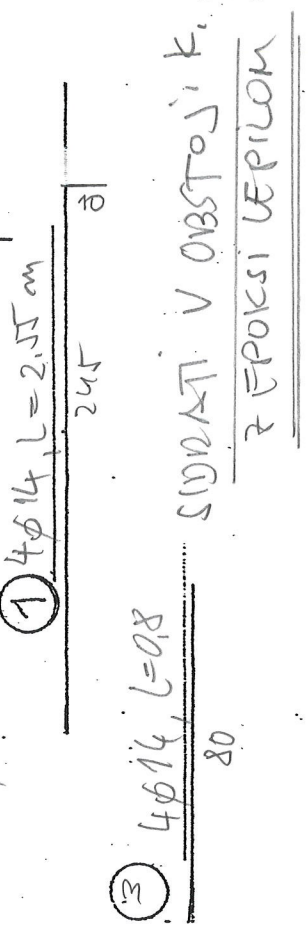
5X2
10X
C25/30 ≤ MB 30 ; RA, MA ⇒ S500

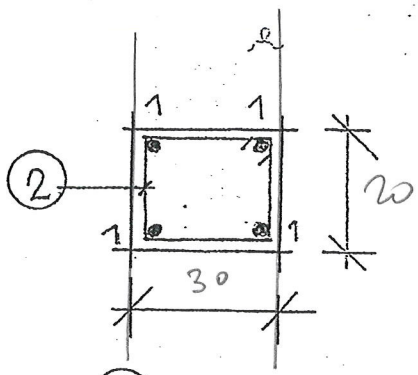
ARMATURA

VERT. VEZ / POZ. (V) — KOM 4

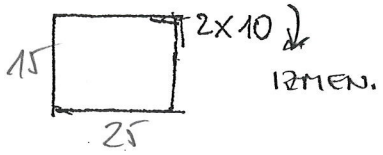
C 25/30, S 500 P







② 18 φ 8, L = 1,0 m¹



ZA 1 KOM !

Poz.	Kom	φ	L	Kg/m¹	Kg φ≤12	Kg φ>12	
1	4	14	2,55	1,25		13,0	
2	18	8	1,00	0,41	7,5		
3	4	14	0,80	1,25		4,0	
4							
5							
6.							
kg					7,5	17,0	ZA 1 KOM
Σ					30,0	68,0	ZA 4 KOM
ΣΣ						98,0	kg

b 1

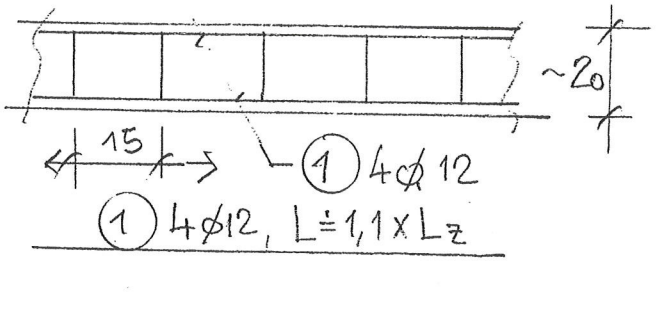
tkom
KONTROLA DIMENZIJ NA OBJEKTU!

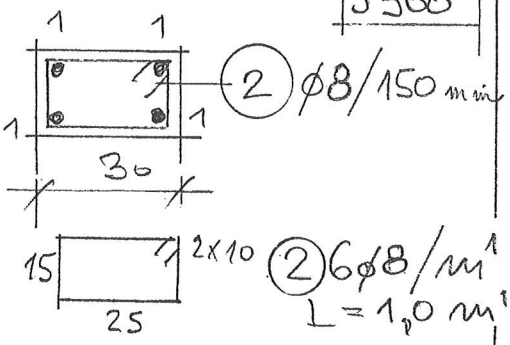
15 X

HORIZONTALNA VEZ 30/20 cm - POZ. HV

C 20/25
S 500

SKUPNA DOLŽINA ~ 12 m'





① 4ϕ12, L ≐ 1,1 × Lz

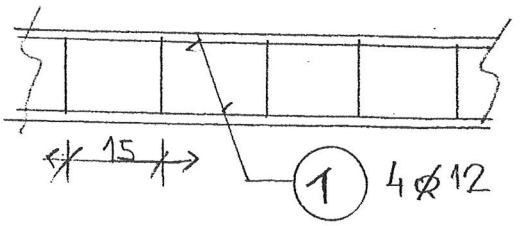
za 1 m'!

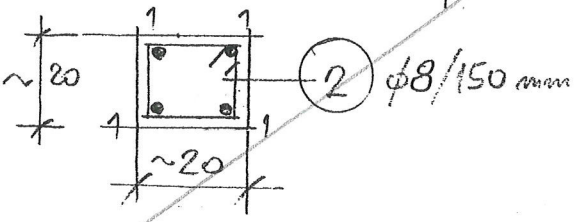
Poz	Kom	ϕ	L	kg/m'	kg ϕ≤12
1	4	12	1,1	0,92	4,0
2	6	8	1,0	0,41	2,5
				kg	6,5 ZA 1 m
				Σ kg	78,0 ZA 12 m' zidu

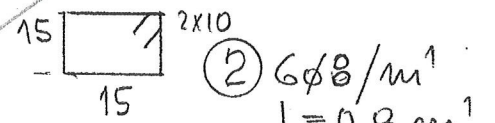
HORIZONTALNA VEZ 20/20 cm - POZ.

C 20/25
S 500

SKUPNA DOLŽINA ~ ____ m'







① 4ϕ12; L ≐ 1,1 × Lz

za 1 m'!

Poz	Kom	ϕ	L	kg/m'	kg ϕ≤12
1	4	12	1,1	0,92	4,0
2	6	8	0,8	0,41	2,0
				kg	6,0 ZA 1 m'
				Σ kg	ZA ____ m' zidu

* KONTROLA MER IN RAZREZ NA OBJEKTU!

10x

IZVLEČEK ARMATURE

S 500

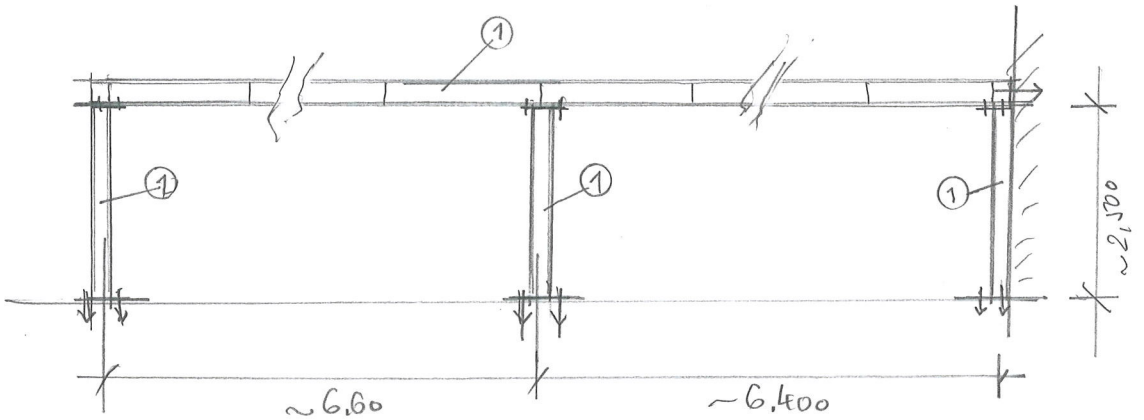
(b) (VRTEC)

POZ.	OPIS		RA	MA
VV	VERT. VEJI		98,0	
HV	HOR. VEZ		78,0	
		Σ	176,0	kg

b3

POZ: (N)1, (S)

JEKLO; S 235



POZ	OZN.	Σ[km]	kg/m	kg
1	HEA 220	21.0	50,5	1.060
—	VEZNI EL.	~10%	OCENA	106 kg
			Σ	1166 kg

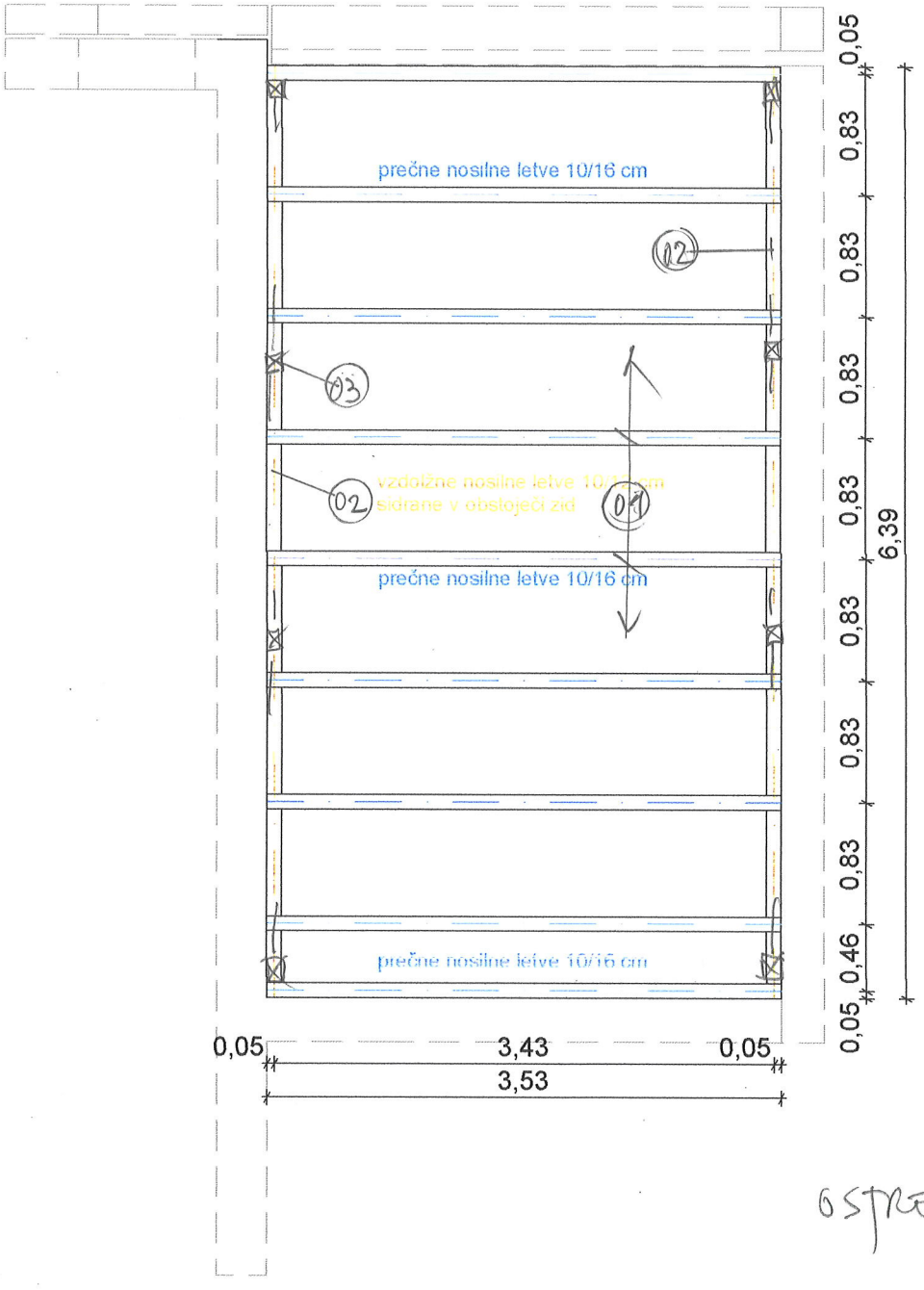
OP.: - VROČE CINKANO
- KONTROLA MER NA OBJEKTU
- DETAJLE PRIPRAVI IZVAJALEC
- SKLADATI V KONSTRUKCIJO (→)!

POZICIJE

SHRAMBA

1:50

tloris ostrešja nova shramba



OSTREŠJE

1:50

SHRAMBA

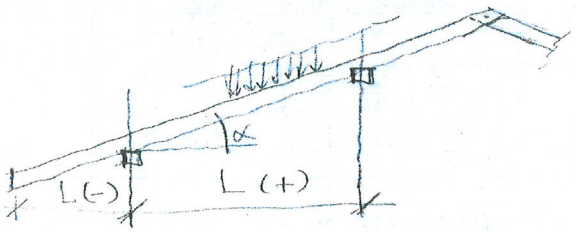
STATIKA

ŠTURM d.o.o. 25.9.2025

DIMENZIONIRANJE LESENIH OSTREŠIJ
(les iglavcev II. kategorije - C24)

OŠT-STRAMBA

Poz. 01 špirovec



razpon (L) (horizontalno)				3,40 m			
razmik (e)				0,90 m			
naklon (α) (do horizontale)				5 °			
višina objekta (z)				6 m			
cona vetra (cv)	tlak vetra	0,43 KN/m²	1	0,00 KN/m²	1,5*0,6	0,00 KN/m²	
kategorija terena			III				
nadmorska višina				320 m			
cona snega	sneg	1,54 KN/m²	A2	1,23 KN/m²	1,5	1,85 KN/m²	
snegobrani?				da			
podstrešje izkoriščeno?				da			
razred uporabnosti				2			
stalna obtežba (poševno)		0,75 KN/m²		0,75 KN/m²	1,35	1,02 KN/m²	
koristna obtežba (vertikalno)				0,10 KN/m²	1,50	0,15 KN/m²	
skupna obtežba				2,09 KN/m²	1,45	3,02 KN/m²	
				(1,88 KN/m)			
upogibni moment						3,92 KNm	
prečna sila						4,62 KN	

neto dimenzije lesa (b/h): **10,0** / **16,0** cm

W_d = 427 cm³ > 358

I_d = 3413 cm⁴ > 2625

MSN

upogib:	$\sigma_{m,d}/f_{m,d}$ =	0,92 / 1,50 KN/cm²	0,61	<	1,0
strig:	$\tau_d/f_{v,d}$ =	0,06 / 0,15 KN/cm²	0,41	<	1,0

MSU

dejanski poves:	w_{inst} =	0,9 cm	<	1,1 (L/300)	0,77	<	1,0
končni poves:	w_{fin} =	1,3 cm	<	1,4 (L/250)	0,92	<	1,0

prečna povezanost špirovca? da

bočna zvrnitev: ni merodajna

$\lambda_{rel,m}$ =

pohodni prostori? ne

vibracije: ni merodajno

f_1 =

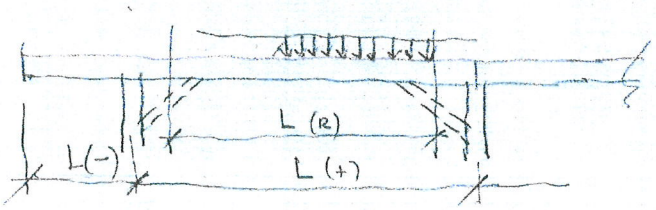
Upoštevano: $k_{mod}(1,2) = 0,8$; $k_{mod}(3) = 0,65$

DIMENZIONIRANJE LESENIH OSTREŠIJ

(les iglavcev II. kategorije - C24)

OŠT-S

Poz. 02 lega + ROČICE !



razpon (L) (horizontalno)	3,00	m
vplivna širina (horizontalno)	1,80	m
naklon (α) (do horizontale)	0	°

POZ 03 / SOHA ! DIM ≥ 16/16 cm

lastna teža lege	0,30 KN/m		0,30	KN/m	1,35	0,41 KN/m
koristna obtežba (vertikalno)			0,10	KN/m	1,50	0,15 KN/m
skupna obtežba			4,16	KN/m	1,44	5,99 KN/m
		(1,66	2,50	KN/m	)	
upogibni moment						6,74 KNm
prečna sila						8,98 KN

neto dimenzije lesa (b/h): **16,0** / **16,0** cm

W_d =	683	cm^3	>	581
I_d =	5461	cm^4	>	4292

MSN

upogib:	$\sigma_{m,d}/f_{m,d}$ =	0,99 / 1,50	KN/cm²	0,66	<	1,0
strig:	$\tau_d/f_{v,d}$ =	0,08 / 0,15	KN/cm²	0,50	<	1,0

MSU

dejanski poves:	w_{inst} =	0,7 cm	<	1,0 (L/300)	0,73	< 1,0
končni poves:	w_{fin} =	1,1 cm	<	1,2 (L/250)	0,89	< 1,0

prečna povezanost lege?		da
bočna zvrnitev	ni merodajna	
$\lambda_{rel,m}$ =		
pohodni prostori?		ne
vibracije	ni merodajno	
f_1 =		

Upoštevano: $k_{mod}(1,2) = 0,8$; $k_{mod}(3) = 0,65$